(14)

PINNACLE STRUCTURES, INC.™

TRANSMITTAL

2665 HIGHWAY 321 BYPASS
PO BOX 1268
CABOT, AR. 72023
PHONE: 501-941-3929
FAX: 501-941-2675

TO: **7B BUILDING & DEVELOPMENT**

" NO HARD COPIES WILL BE MAILED "

ATTN: Derrick Merchant

DATE: **7/16/2015**

JOB #: **151400A**

WE ARE SENDING YOU THE FOLLOWING ITEMS:

____ SETS OF ANCHOR BOLT SETTING PLANS	[ ] WITH CERT. SHEET	[ ] SEALED
____ SETS OF **REVISED** ANCHOR BOLT SETTING PLANS	[ ] WITH CERT. SHEET	[ ] SEALED
3 SETS OF PERMIT DRAWINGS NOT FOR CONSTRUCTION	[X] WITH CERT. SHEET	[X] SEALED
____ SETS OF APPROVAL DRAWINGS NOT FOR CONSTRUCTION	[ ] WITH CERT. SHEET	[ ] SEALED
____ SETS OF FINAL DRAWINGS FOR CONSTRUCTION	[ ] WITH CERT. SHEET	[ ] SEALED
____ SETS OF ENGINEERING CALCULATIONS		[ ] SEALED
____ SETS OF _____		

[X] EMAIL DWGS TO: **derrick@7bdev.com**
trey@7bdev.com

CC: **Josh**

NOTES: **Email only.**

Include PDF copy of B-lok roof manual.

PINNACLE STRUCTURES, INC
BY: **Josh Fairchild**

[ ] US MAIL [ ] 2ND DAY [ ] OVERNIGHT [ ] CUSTOMER PICK-UP

PINNACLE STRUCTURES, INC.™
P. O. Box 1268
Cabot, Arkansas 72023
501-941-3929

ACCREDITED
MBMA METAL BUILDING MANUFACTURERS ASSOCIATION
AC472 ACCREDITED
MB - 103

Customer: 7B Building & Development
13105 CR 1820
Lubbock, TX 79424

Pinnacle Job # : 151400
Project : Champion Express
Project Location : San Angelo, TX (Tom Green Co.)
Project Description: 38' W x 127' L x 12' EH – 8:12 Bldg A
31'-1.5" W x 21' L x 12' EH - 8:12 Tower Structures

This is to certify that the above referenced building or buildings has been designed in accord with the American Institute of Steel Construction (AISC-13th edition) , American Iron and Steel Institute (AISI-NAUS-2007) specifications and the IBC-2009 building code. The building and its components are designed and produced in an IAS approved facility by an IAS AC472 accredited manufacturer. Pinnacle Structures, Inc. is an IAS AC472 accredited manufacturer. Members are designed for the following loads specified on your Purchase Order:

Design Loads:

IBC Building Occupancy Category: II - Standard Occupancy

1) 4 PSF Metal Building Roof Dead Load
2) 3* PSF Collateral Load *
3) 20 PSF Secondary Live Load
4) 20 PSF Primary Live Load *(Reducible per Code)*
5) 5 PSF Ground Snow Load
6) 3.18 PSF Roof Snow Load
   C_e = 1.0 C_t = 1.0 C_s = 0.91 I_s = 1.0
7) 90 MPH Wind Speed/Load (3 sec gust)[1]
   I_w = 1.00 Exposure = C
8) Seismic: I_E = 1.0 *Equivalent lateral-force procedure*
   S_S = 0.06 S_{DS} = 0.07
   S_1 = 0.03 S_{D1} = 0.04
   Site Class:D
   Seismic Design Category: A
9) Crane: Quantity:
   Type: N/A
   Capacity: N/A Ton
   Max. wheel load: N/A Kips
10) Mezzanine:
   N/A
   Dead Load: N/A PSF
   Live Load: N/A PSF
11) Other:
   N/A

TRANSVERSE DIRECTION (Moment Frames)
Ordinary Steel Moment Frames (R=3.5, Ωo=3.0, Cd=3.0***)

END WALLS
Ordinary Steel Moment Frames (R=3.5, Ωo=3.0, Cd=3.0***) Left Endwall: Non-Expandable Rigid Frame
Ordinary Steel Moment Frames (R=3.5, Ωo=3.0, Cd=3.0***) Right Endwall: Non-Expandable Rigid Frame

LONGITUDINAL DIRECTION
Ordinary Steel Concentrically Braced Frames (R=3.25,Ωo=2,Cd=3.25***) Front Sidewall: Rod Brace
Ordinary Steel Concentrically Braced Frames (R=3.25,Ωo=2,Cd=3.25***) Back Sidewall: Rod Brace

[1] Note: This project is designed as an Enclosed (GCp=±0.18) Building. Accessories (doors, windows, etc.) by others must be designed as "components and cladding" per the referenced Building Code.

*Note: This project is designed for this collateral loading. NO additional loads shall be attached to the Pinnacle structure that will exceed this loading. All loads suspended from the purlins must be attached to the purlin webs and not the purlin flanges. Under NO circumstances are the purlin flanges to be modified by cutting, drilling or bending of flange or lip.

***Note: The shown values of C_d were used in determination of PΔ effects.
Per ASCE7-05 Sec. 12.12.1 (Table 12.12-1) , this structure is designed for story drift requirements. Any interior ceilings, partitions, walls and other attached elements must be detailed to accommodate story drifts. The seismic drift limit used is H/143.

This Letter of Certification applies solely to the structural framing and its component parts as furnished by Pinnacle Structures, Inc. and specifically excludes any foundation, masonry, or general contract work to include erection certification.

The design and Certification for this project is in accord with the provisions and loads specified on the building order. The buyer is responsible to verify specified loads are in compliance with the local regulatory authorities.

The undersigned is not the engineer of record for the overall project.

Sincerely,

STATE OF TEXAS
STEPHEN G. OWEN
117631
LICENSED
PROFESSIONAL ENGINEER

Pinnacle Structures, Inc.
Texas Registered Engineering Firm F-10353
2665 Hwy 321 Bypass, P.O. Box 1268
Cabot AR 72023

GENERAL NOTES

1. This structure has been designed in accordance with the 2007 AISI NAUS Cold Formed Steel Design Manual and the AISC (13th Edition, ASD) Steel Construction Manual.

2. Fabrication shall be accordance with Pinnacle Standards in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1".

3. Materials	ASTM Designation	Minimum Yield
Hot Rolled Angle	A36	Fy = 36 ksi
Structural Steel Plate	A572, A529, A1011	Fy = 55 ksi
Cold Formed Shapes	A1011/(A653 Galvanized)	Fy = 55 ksi
Cable Bracing	A475	Ex. High Strength
Roof & Wall Sheeting	A792 29 and 26 GA	80 ksi, Class 1
	A792 24 and 22 GA	50 ksi, Class 2
High Strength Bolts	A325-Group A/(A490-Group B)	
Pipe	A53, Gr. B	Fy = 35 ksi
Round Structural Tubing	A500, Gr. B	Fy = 42 ksi
Shaped Structural Tubing	A500, Gr. B	Fy = 46 ksi
Hot Rolled Shapes	A572, A992, A529 Gr. 50	Fy = 50 ksi
Hot Rolled Shapes	A36	Fy = 36 ksi

4. Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification TT-P-636 and is maroon oxide in color. This paint is not intended for long term exposure to the elements. Pinnacle Structures, Inc. is not responsible for any deterioration of the shop primer as a result of improper handling or storage. Pinnacle will not be responsible for any field applied paint and or coatings. (Section 6.5 AISC code of Standard Practice, 9th Edition).

5. Bolts for the construction of Pinnacle Structures, Inc. material shall be as follows:
   - All secondary member connections – 1/2" x 1 1/4" A307 unless noted
   - Bearing frame endwall connections – A325
   - Main frame connections – A325 as shown on drawings

6. Connections Using High Strength Structural Bolts:

All high strength bolts are A325-N, unless noted otherwise. High strength structural bolts are supplied without washers, unless noted otherwise. Bolt length shall be such that the end of the bolt extends beyond or is at least flush with the outer face of the nut, when properly installed. All bolted connections, unless noted, are designed as bearing type connection with bolt threads not excluded from the shear plane.

A325-N High Strength Structural Bolts:
Snug-Tightened connections are permitted with A325-N bolts, except for these cases:
- Where crane beams and rigid frame connections in crane buildings are present
- In Slip-Critical Connections
- If noted in the erection drawings otherwise

For these exceptions, Turn-of-the-Nut method must be used.

A490 High Strength Structural Bolts:
A490 structural bolts shall be tightened using the Turn-of-the-Nut method. Snug-Tightened connections are not permitted with A490 bolts.

Tightening Methods:

Snug-Tightened Joint: A condition in which the tightness that exists when all of the plies in a connection have been pulled into firm contact by the bolts in the joint and all of the bolts in the joint have been tightened sufficiently to prevent the removal of the nuts without the use of a wrench, in accordance with the 14th Edition of AISC "Specification for Structural Joints Using High-Strength Bolts", per Section 8.1.

Turn-of-the-Nut method in is to be performed in accordance with the 14th Edition AISC "Specification for Structural Joints Using High-Strength Bolts" per Section 8.2.1.

7. All Bracing shown and provided by Pinnacle for this building is required for transferring building loads to the foundation and shall be installed by the erector as a permanent part of the structure. Cable/Rod bracing is designed for structural loads only and is not designed to plumb the building. The cable/rod bracing shall be taut, tighten to remove sag only. Bracing shall not be over-tighten. If additional bracing is required for stability during erection, it shall be the erectors responsibility to determine the amount of such bracing and to procure and install as necessary.

8. Soil profile type is determined by the foundation Engineer per local code.

9. Building Codes Require Consideration of Snow Surcharges for Any Lower Roof of a Structure Located within 20 Feet of a Higher Structure. Information Supplied to Pinnacle Structures Does Not Indicate the Presence of a Shadowing Structure within this 20 Foot Envelope. Therefore Snow Surcharges Have Not Been Considered in this Design Unless Noted Otherwise.

PINNACLE™ STRUCTURES, INC.

2665 Bill Foster Mem. Hwy, P.O. Box 1268
Cabot, AR 72023
Phone: (501) 941-3929 or (800) 201-1534
Fax: (501) 941-2675

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION
AC472 ACCREDITED
MB-103

IAS ACCREDITED

DESIGN REQUIREMENTS

Building Code: IBC 2009
IBC Building Category: II – Standard Occupancy
Metal Building Dead Load: 4.0 psf
Collateral Load: 3 * psf
Secondary Live Load: 20 psf
Primary Live Load: 20 psf (REDUCIBLE PER CODE)
Ground Snow Load: 5 psf
Roof Snow Load: 3.18 psf
Ce= 1.0 Ct= 1.0 Cs= 0.91 Is = 1.0
Wind Speed/Load: 90 mph (3 Second Gust)[1]
Iw = 1.0 Exposure = C

Seismic: *EQUIVALENT LATERAL FORCE PROCEDURE*
Seismic Coefficient: I_E= 1.0 S_S = 0.06 S_{DS}= 0.07
S_1 = 0.03 S_{D1}= 0.04

Site Class: D
Seismic Design Category: A

TRANSVERSE DIRECTION (Moment Frames)
Ordinary Steel Moment Frames (R=3.5, Ω_0=3.0, Cd=3.0***)

END WALLS
Ordinary Steel Moment Frames (R=3.5, Ω_0=3.0, Cd=3.0***) Left Endwall: Non-Expandable Rigid Frame
Ordinary Steel Moment Frames (R=3.5, Ω_0=3.0, Cd=3.0***) Right Endwall: Non-Expandable Rigid Frame

LONGITUDINAL DIRECTION
Ordinary Steel Concentrically Braced Frames (R=3.25, Ω_0=2.0, Cd=3.25***) Front Sidewall: Rod Brace
Ordinary Steel Concentrically Braced Frames (R=3.25, Ω_0=2.0, Cd=3.25***) Back Sidewall: Rod Brace

Pinnacle Structures, Inc.
Texas Registered Engineering Firm F-10353
2665 Hwy 321 Bypass, P.O. Box 1268
Cabot AR 72023

Crane:
Type: N/A
Capacity: N/A Ton
Max. wheel load not including impact N/A Kips

Mezzanine:
Dead Load: N/A psf
Live Load: N/A psf

Snow Drift:
Pd: N/A psf
Wd: N/A feet

Other: N/A

DRAWING PACKAGE FOR:

JOB NUMBER: 151400A

CUSTOMER: 7B BUILDING & DEVELOPMENT

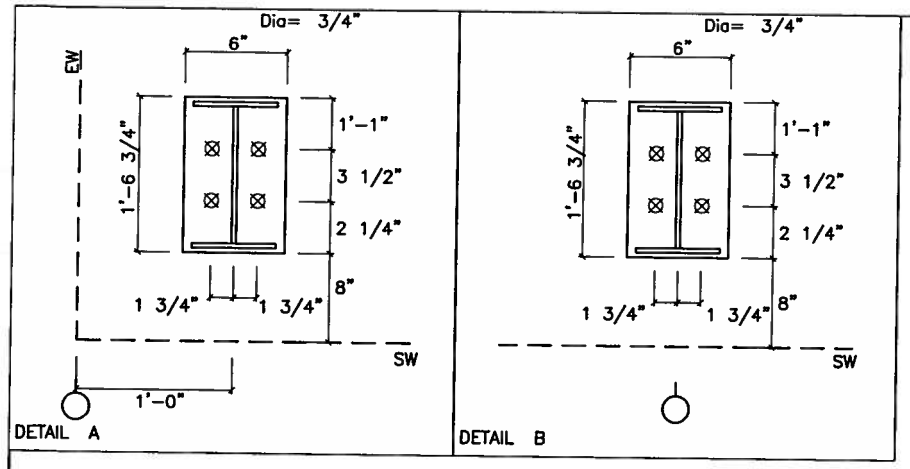
PROJECT: CHAMPION EXPRESS CAR WASH

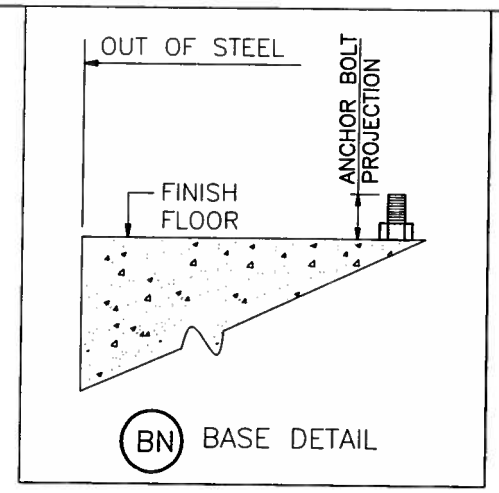
JOBSITE: SAN ANGELO, TX

BUILDING SIZE: 38' x 127' x 12' EH (8:12)

* This project is designed for this collateral loading. NO additional loads shall be attached to the Pinnacle structure that will exceed this loading. All loads suspended from the purlins must be attached to the purlin webs and not the purlin flanges. Under NO circumstances are the purlin flanges to be modified by cutting, drilling or bending of flange or lip.

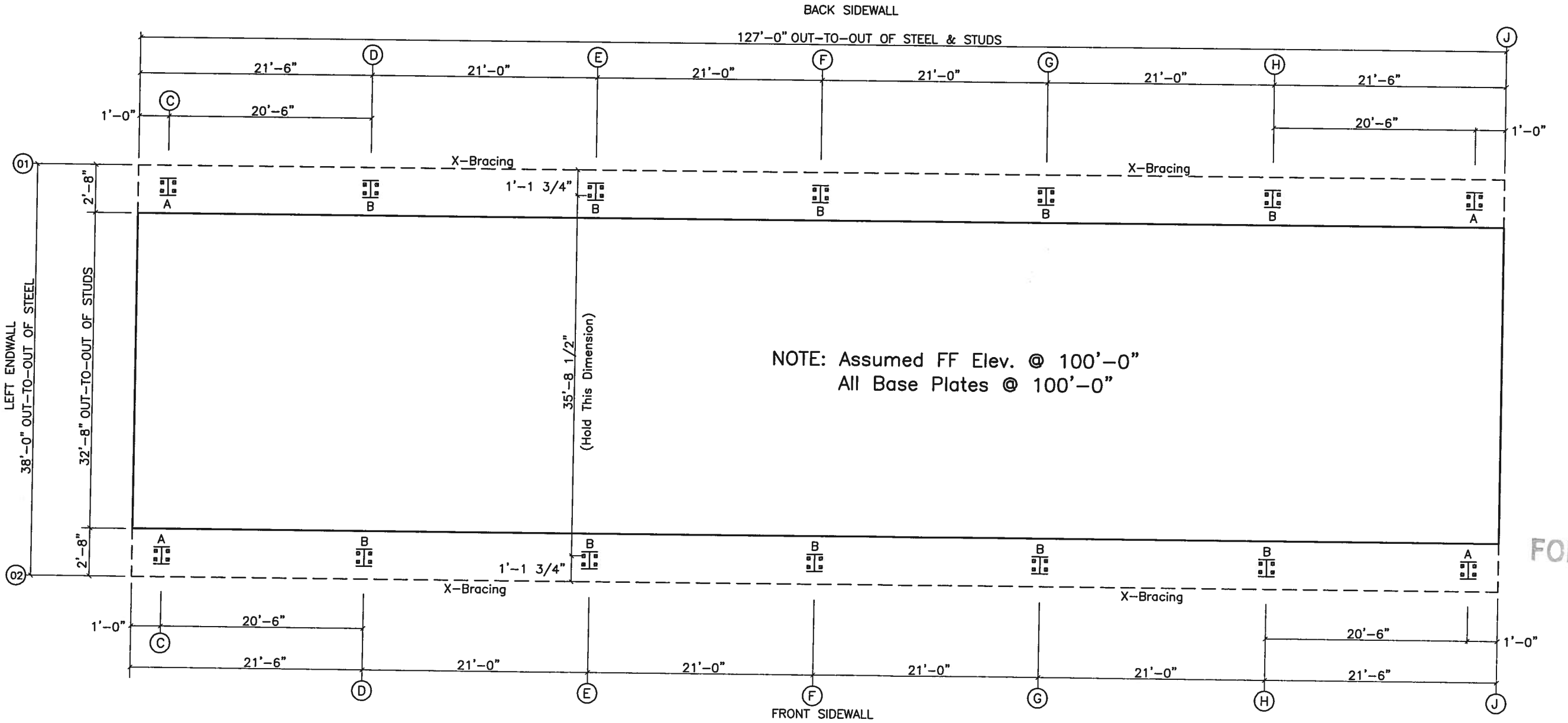
*/***/1 SEE LETTER OF CERTIFICATION.





ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
⊠ 56	Frame	3/4"	A307	2.50



ANCHOR BOLT PLAN

FOR PERMITS ONLY

GENERAL NOTES

These Drawings Are Not To Scale.
Pinnacle's Steel Line Is Shown. A Sheeting Notch Or Brick Ledge, If Used, Must Be Added To Determine The Out Of Concrete.

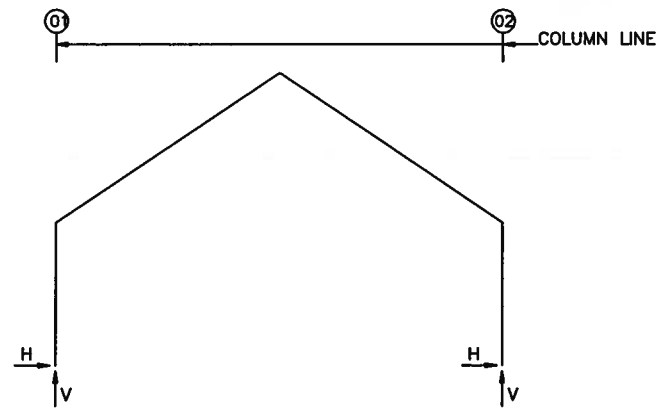
ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ANCHOR BOLT PLAN
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet F01	Issue P

FRAME LINES: C D E F G H J



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
C	01	3	3.3	4.7	4	−2.6	0.5	4	0.750	6.000	18.75	0.375	0.0
		1	1.9	5.9	8	0.7	−2.7						
C	02	5	2.6	0.5	2	−3.3	4.7	4	0.750	6.000	18.75	0.375	0.0
		1	−1.9	5.9	9	−0.7	−2.7						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
D*	01	3	5.2	7.7	4	−3.7	1.1	4	0.750	6.000	18.75	0.375	0.0
		1	3.2	9.8	6	1.1	−6.0						
D*	02	5	3.7	1.1	2	−5.2	7.7	4	0.750	6.000	18.75	0.375	0.0
		1	−3.2	9.8	7	−1.1	−6.0						

D* Frame lines: D E F G H

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
J	01	3	3.3	4.7	4	−2.6	0.5	4	0.750	6.000	18.75	0.375	0.0
		1	1.9	5.9	8	0.7	−2.7						
J	02	5	2.6	0.5	2	−3.3	4.7	4	0.750	6.000	18.75	0.375	0.0
		1	−1.9	5.9	9	−0.7	−2.7						

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
C	01	0.5	1.8	0.3	0.9	1.1	3.2	0.3	0.8	−2.7	−1.9	2.3	−0.5
C	02	−0.5	1.8	−0.3	0.9	−1.1	3.2	−0.3	0.8	−2.3	−0.5	2.7	−1.9

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
C	01	−2.9	−0.5	2.1	0.9	0.2	−3.4	−0.1	−2.0	−0.3	−0.2	0.3	0.2
C	02	−2.1	0.9	2.9	−0.5	−0.2	−3.4	0.1	−2.0	−0.3	0.2	0.3	−0.2

Frame Line	Column Line	LWIND1_L2E Horiz	LWIND1_L2E Vert	LWIND1_R2E Horiz	LWIND1_R2E Vert	LWIND2_L2E Horiz	LWIND2_L2E Vert	LWIND2_R2E Horiz	LWIND2_R2E Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
C	01	0.2	−0.3	−0.2	−0.2	0.2	−0.3	−0.2	−0.2	0.3	0.9	0.3	0.6
C	02	0.2	−0.2	−0.2	−0.3	0.2	−0.2	−0.2	−0.3	−0.3	0.6	−0.3	0.9

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
D*	01	0.9	2.8	0.5	1.6	1.8	5.4	0.5	1.4	−3.8	−3.1	3.3	−1.0
D*	02	−0.9	2.8	−0.5	1.6	−1.8	5.4	−0.5	1.4	−3.3	−1.0	3.8	−3.1

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
D*	01	−4.2	−0.6	2.9	1.5	0.3	−7.0	−0.1	−4.5	−0.3	−0.2	0.3	0.2
D*	02	−2.9	1.5	4.2	−0.6	−0.3	−7.0	0.1	−4.5	−0.3	0.2	0.3	−0.2

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	LWIND1_L2E Horiz	LWIND1_L2E Vert	LWIND1_R2E Horiz	LWIND1_R2E Vert	LWIND2_L2E Horiz	LWIND2_L2E Vert	LWIND2_R2E Horiz	LWIND2_R2E Vert	F2UNB_SL_L Horiz	F2UNB_SL_L Vert
D*	01	0.0	−0.5	0.3	−0.6	−0.3	−0.3	0.3	−0.6	−0.3	−0.3	0.5	1.5
D*	02	0.0	−0.5	0.3	−0.3	−0.3	−0.6	0.3	−0.3	−0.3	−0.6	−0.5	1.0

Frame Line	Column Line	F2UNB_SL_R Horiz	F2UNB_SL_R Vert
D*	01	0.5	1.0
D*	02	−0.5	1.5

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
J	01	0.5	1.8	0.3	0.9	1.1	3.2	0.3	0.8	−2.7	−1.9	2.3	−0.5
J	02	−0.5	1.8	−0.3	0.9	−1.1	3.2	−0.3	0.8	−2.3	−0.5	2.7	−1.9

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
J	01	−2.9	−0.5	2.1	0.9	0.2	−3.4	−0.1	−2.0	−0.3	−0.2	0.3	0.2
J	02	−2.1	0.9	2.9	−0.5	−0.2	−3.4	0.1	−2.0	−0.3	0.2	0.3	−0.2

Frame Line	Column Line	LWIND1_L2E Horiz	LWIND1_L2E Vert	LWIND1_R2E Horiz	LWIND1_R2E Vert	LWIND2_L2E Horiz	LWIND2_L2E Vert	LWIND2_R2E Horiz	LWIND2_R2E Vert	F3UNB_SL_L Horiz	F3UNB_SL_L Vert	F3UNB_SL_R Horiz	F3UNB_SL_R Vert
J	01	0.2	−0.3	−0.2	−0.2	0.2	−0.3	−0.2	−0.2	0.3	0.9	0.3	0.6
J	02	0.2	−0.2	−0.2	−0.3	0.2	−0.2	−0.2	−0.3	−0.3	0.6	−0.3	0.9

D* Frame lines: D E F G H

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions (k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Seis	Note
L_EW	C								(h)
F_SW	02	D,E	1.2	0.6	0.9	0.4			
		G,H	1.2	0.6	0.9	0.4			
R_EW	J								(h)
B_SW	01	H,G	1.2	0.6	0.9	0.4			
		E,D	1.2	0.6	0.9	0.4			

(h)Rigid frame at endwall

GENERAL NOTES

1. ANCHOR BOLTS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
2. FOUNDATION DESIGN AND ANCHOR BOLT LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
3. THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR BOLT SUMMARY TABLE REPORTS THE BOLT DIAMETERS.
4. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
5. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/− 1/8".

FOR PERMITS ONLY

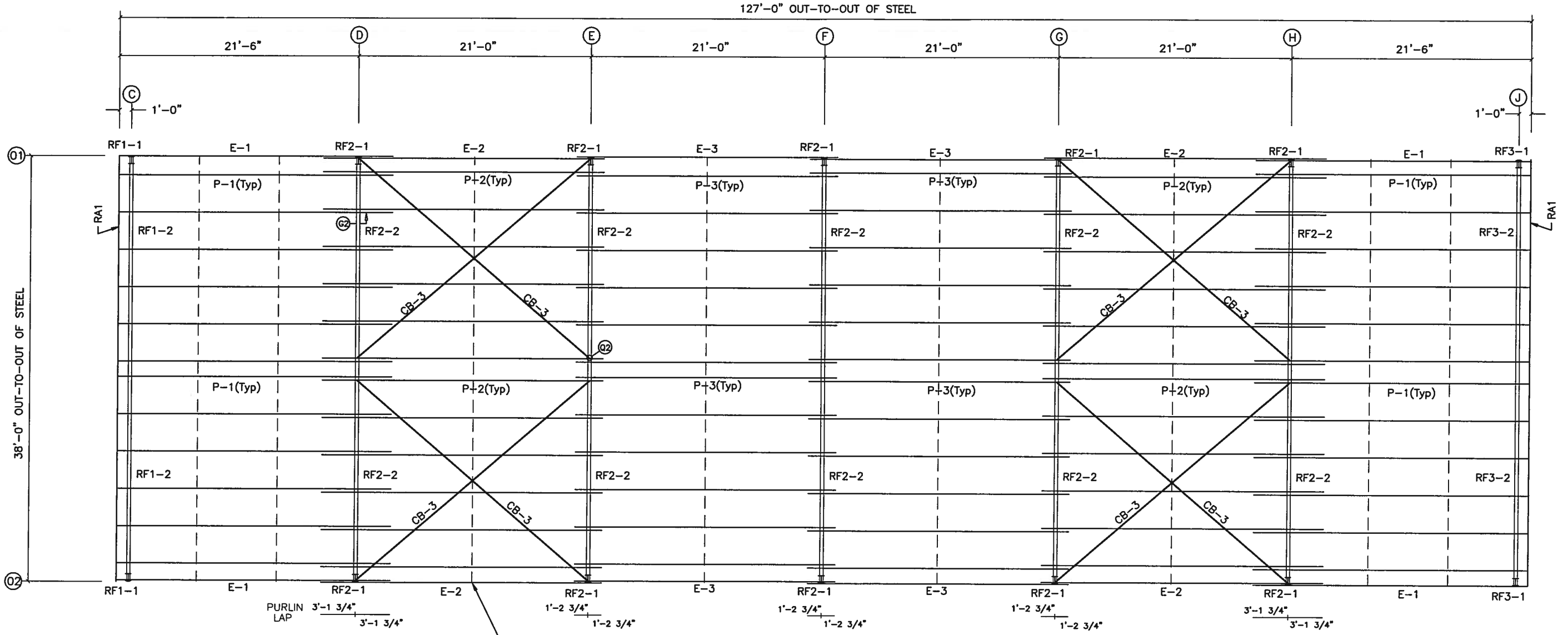
ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE TRU STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

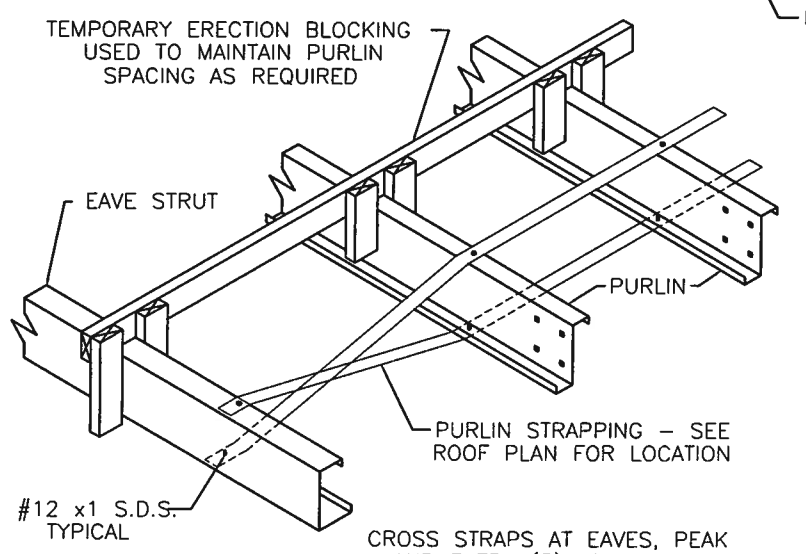
DESCRIPTION:	ANCHOR BOLT REACTIONS
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet F02	Issue P

NOTE:
Alternate Arrows ▽-△
Up And Down From Bay
To Bay For Purlins To Lap.



ROOF FRAMING PLAN



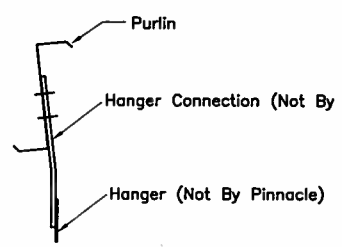
ROOF STRAPPING ISOMETRIC

FOR PERMITS ONLY

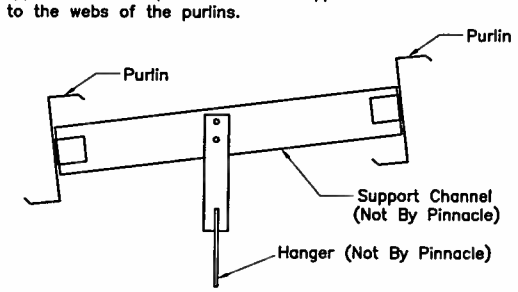
COLLATERAL LOAD NOTE:

Roof purlin has been designed for the collateral load listed on the cover. The total applied loads due to ceiling panels, ducts, sprinkler distribution lines, electrical equipment, conduit, fireproofing, other piping or mechanical loads cannot exceed this maximum uniform load. Pinnacle Structures, Inc. is not responsible for lateral or longitudinal bracing of suspended members subject to lateral seismic or wind loading.

Loads supported directly from the purlins must have connections through the web of the purlin.



Loads supported between purlins must be supported such that the loads are applied to the webs of the purlins.

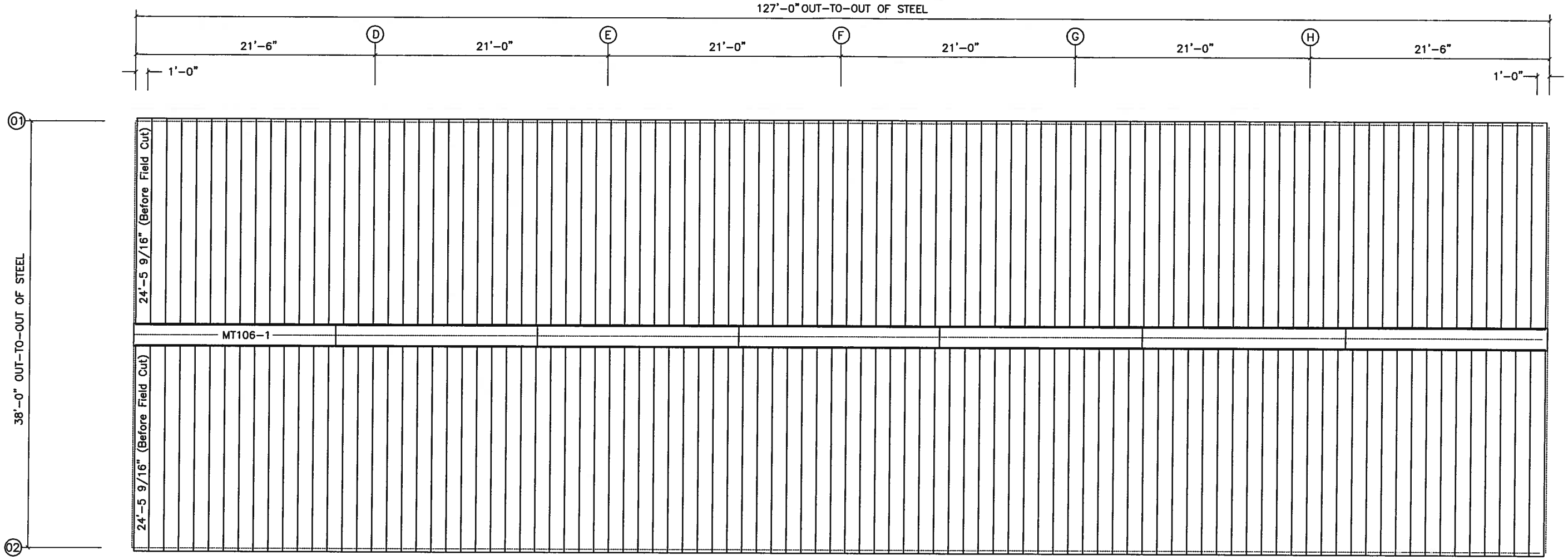


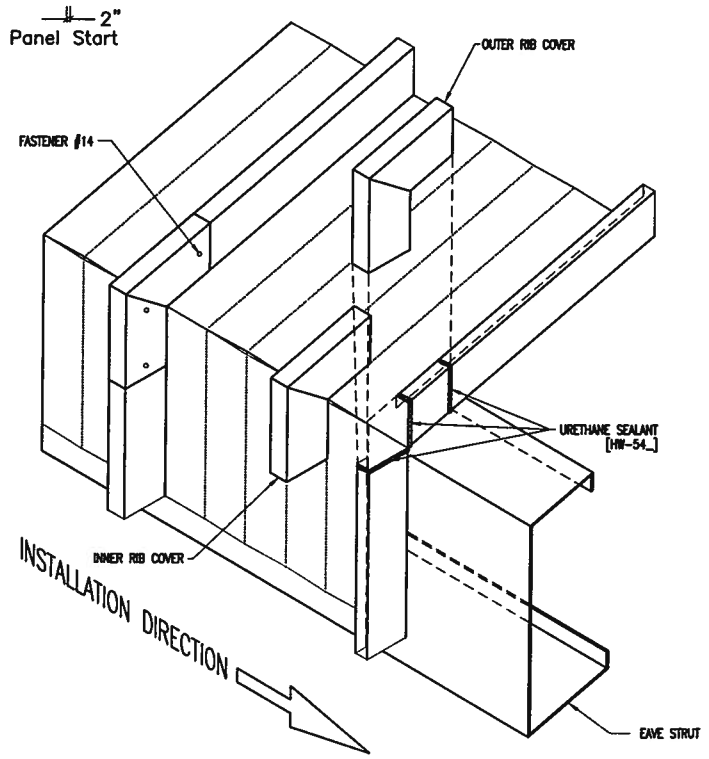
1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7 /15/15	

PINNACLE 1988
STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ROOF FRAMING
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

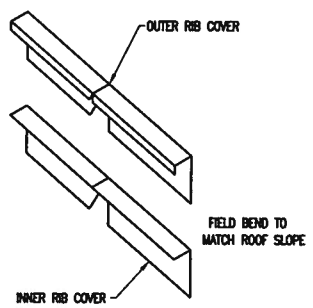
Detailer	JWF	Checker		Designer	
Job No.	151400A	Sheet	E01	Issue	P





ISOMETRIC BATTENLOK ROOF TO WALL TRANSITION

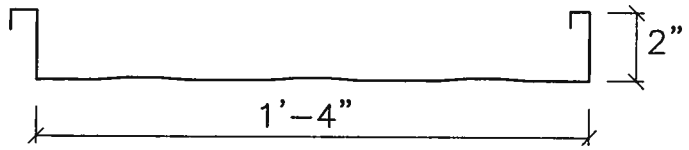
TRANSITION FUNCTIONS AS A FIXED EAVE CONDITION, ROOF CLIPS ABOVE AND BELOW MAY BE FLOATING, FIXED OR UTILITY.



ROOF SHEETING PLAN

PANELS: 24 Ga. BattenLok
COLOR: Brite Red

2"
Panel Start



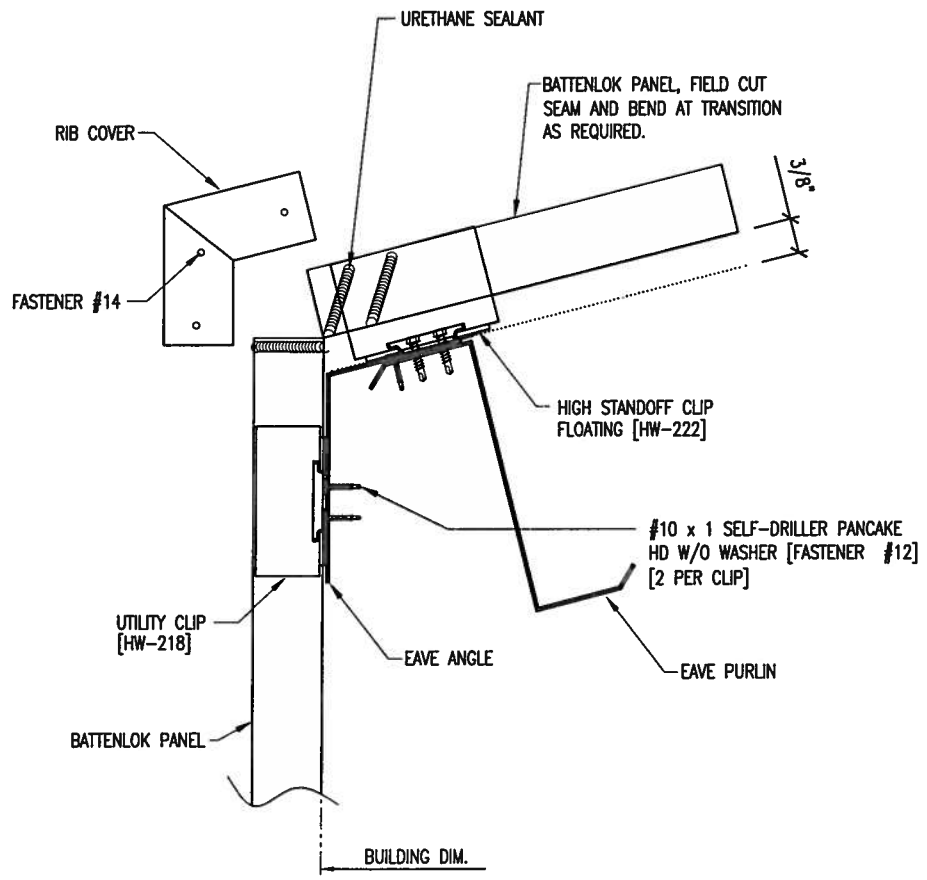
BattenLok Profile

1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

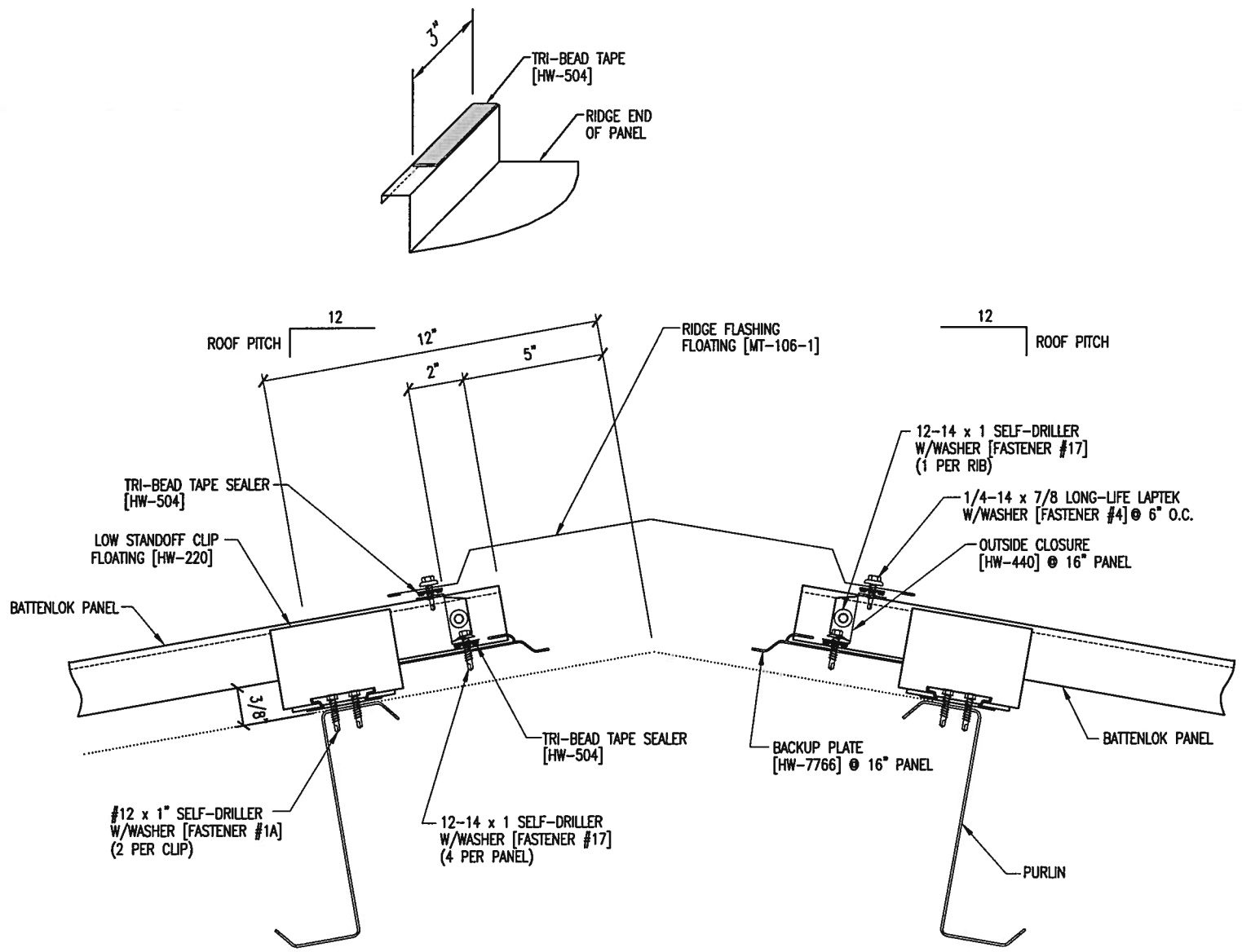
DESCRIPTION:	ROOF SHEETING
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E02	Issue P



SECTION THROUGH ROOF TO WALL TRANSITION

SNOW RETENTION DEVICES, WHERE APPLICABLE, ARE BY OTHERS.



SECTION THROUGH RIDGE

FOR PERMITS ONLY

1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

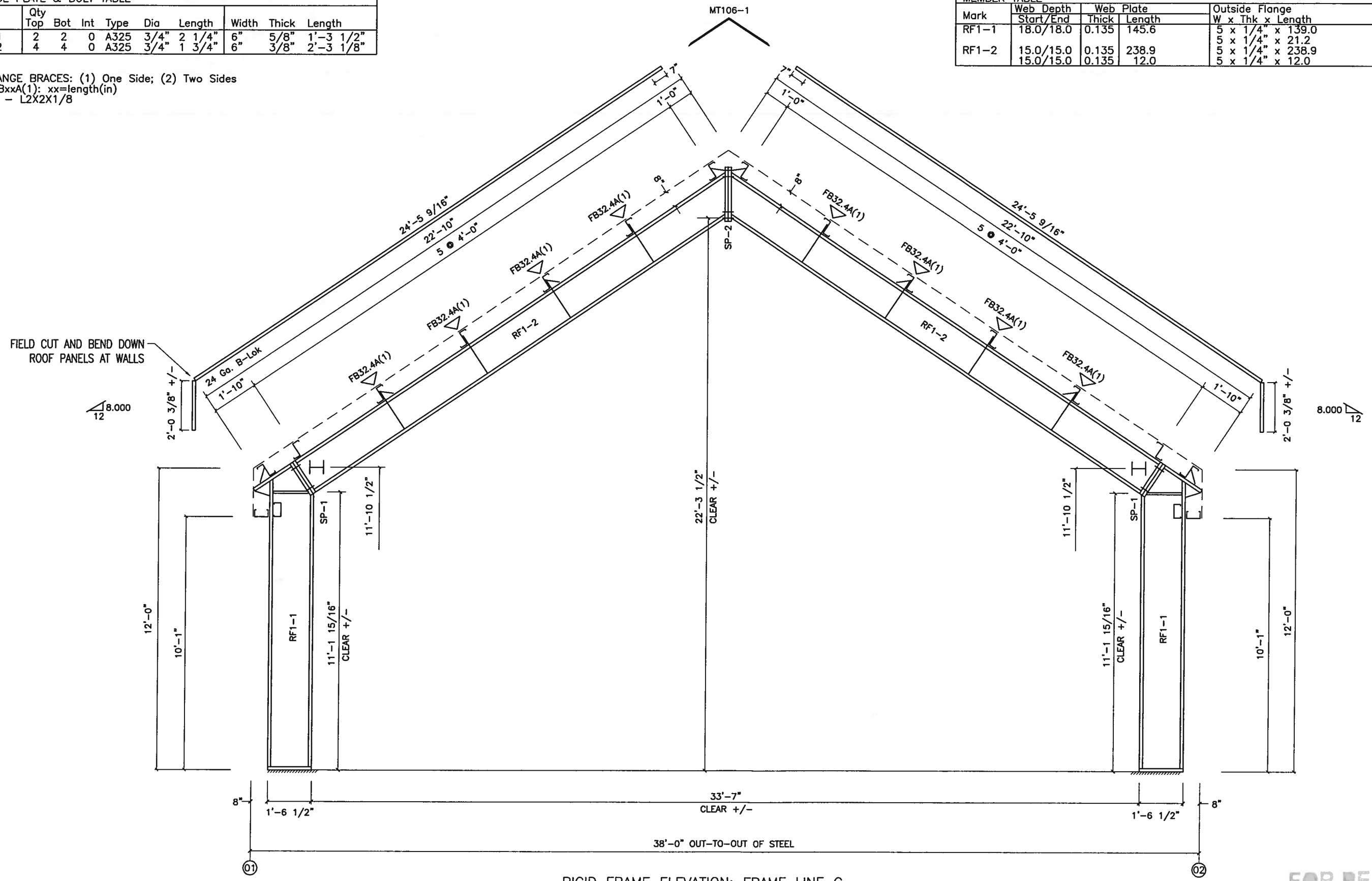
DESCRIPTION:	ROOF SHEETING DETAILS
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E03	Issue P

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	2	2	0	A325	3/4"	2 1/4"	6"	5/8"	1'-3 1/2"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	2'-3 1/8"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	18.0/18.0	0.135	145.6	5 x 1/4" x 139.0 5 x 1/4" x 21.2	5 x 1/4" x 132.9
RF1-2	15.0/15.0 15.0/15.0	0.135 0.135	238.9 12.0	5 x 1/4" x 238.9 5 x 1/4" x 12.0	5 x 1/4" x 228.8 5 x 1/4" x 12.0

FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2X2X1/8



RIGID FRAME ELEVATION: FRAME LINE C

FOR PERMITS ONLY

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

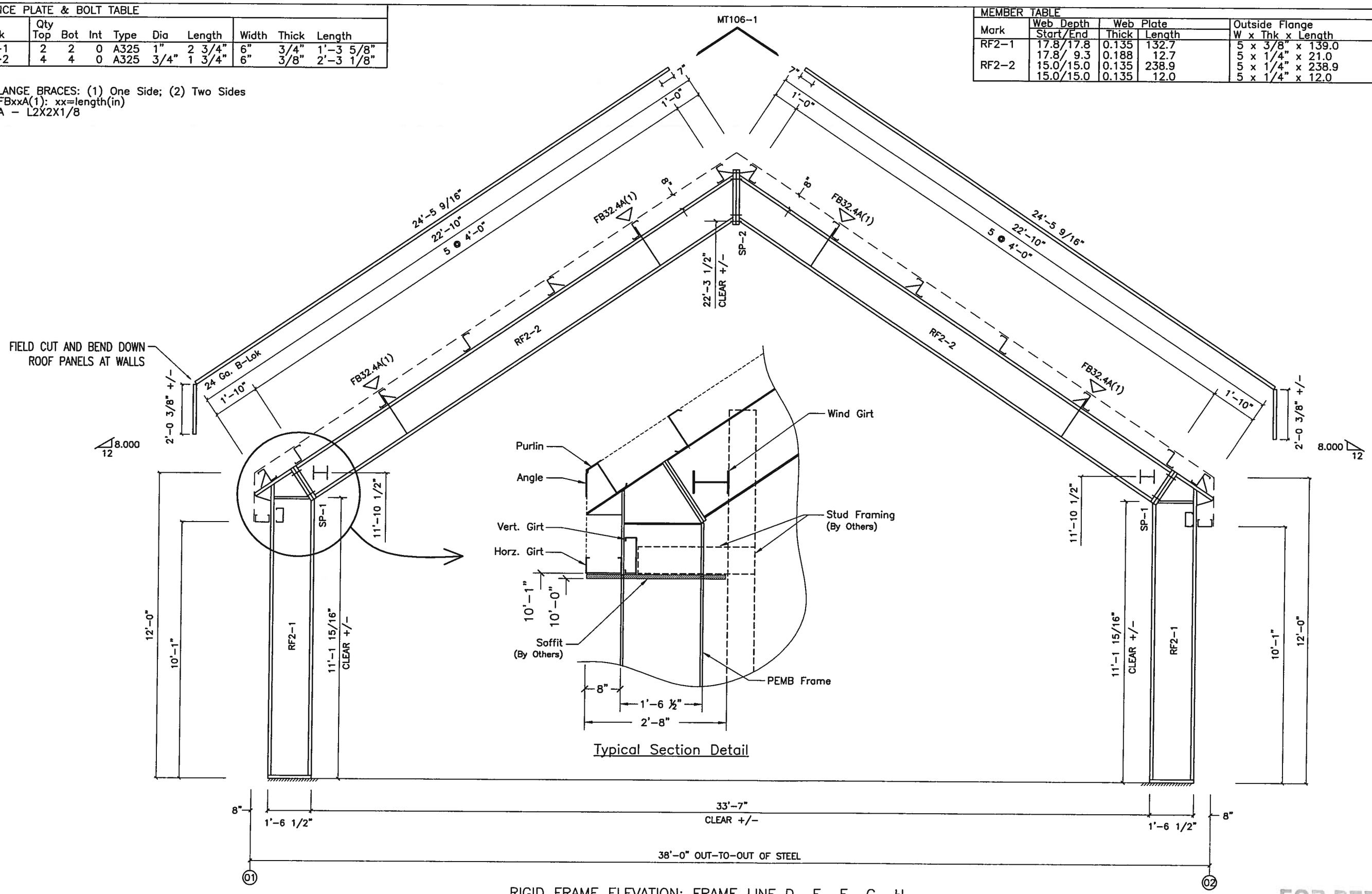
DESCRIPTION:	RIGID FRAME ELEVATION
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E04	Issue P

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	2	2	0	A325	1"	2 3/4"	6"	3/4"	1'-3 5/8"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	2'-3 1/8"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF2-1	17.8/17.8	0.135	132.7	5 x 3/8" x 139.0	5 x 3/8" x 132.7
	17.8/ 9.3	0.188	12.7	5 x 1/4" x 21.0	
RF2-2	15.0/15.0	0.135	238.9	5 x 1/4" x 238.9	5 x 5/16" x 130.9
	15.0/15.0	0.135	12.0	5 x 1/4" x 12.0	5 x 1/4" x 109.8

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2X2X1/8



Typical Section Detail

RIGID FRAME ELEVATION: FRAME LINE D E F G H

FOR PERMITS ONLY

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

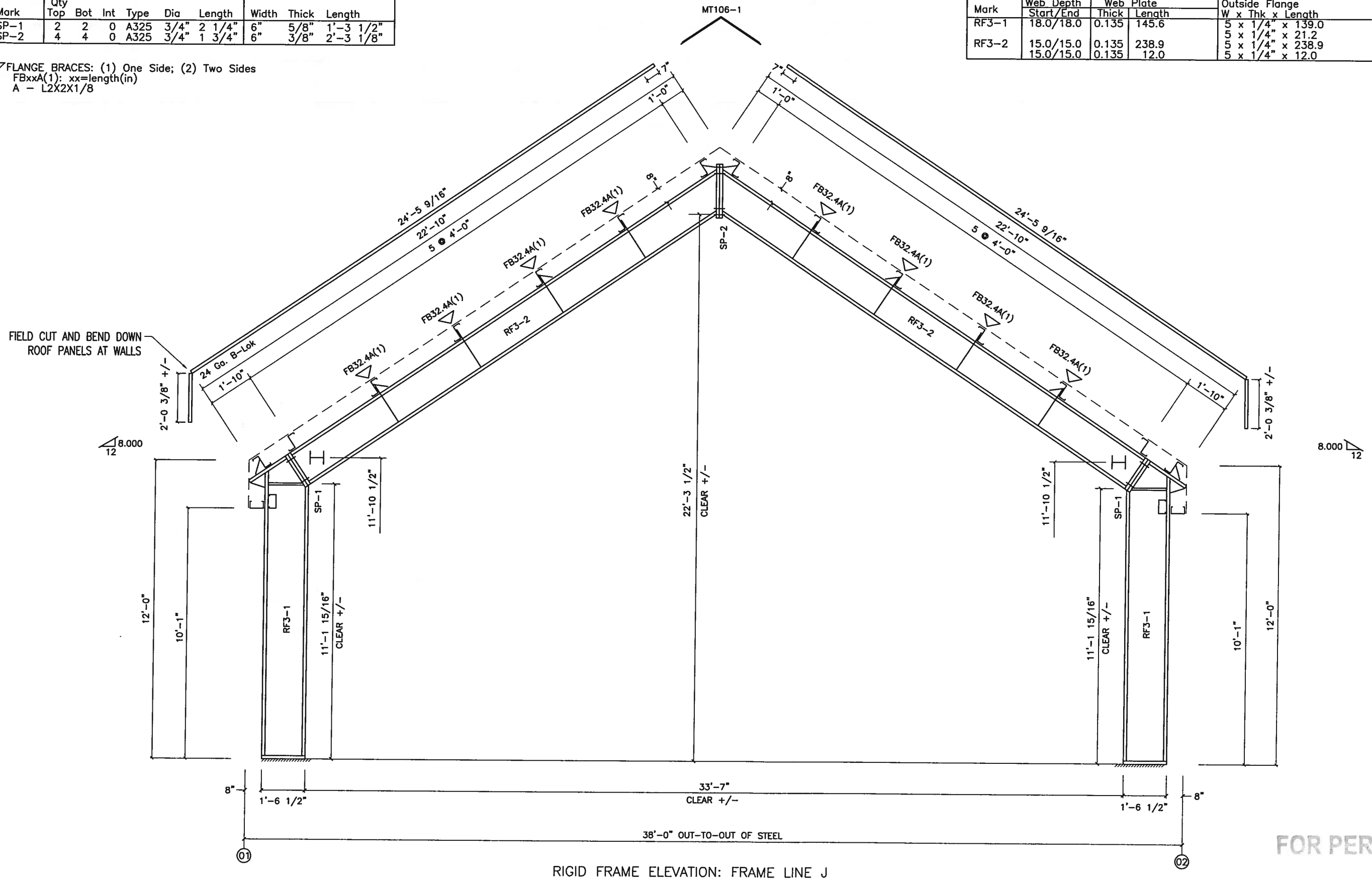
DESCRIPTION:	RIGID FRAME ELEVATION
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E05	Issue P

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	2	2	0	A325	3/4"	2 1/4"	6"	5/8"	1'-3 1/2"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	2'-3 1/8"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF3-1	18.0/18.0	0.135	145.6	5 x 1/4" x 139.0 5 x 1/4" x 21.2	5 x 1/4" x 132.9
RF3-2	15.0/15.0 15.0/15.0	0.135 0.135	238.9 12.0	5 x 1/4" x 238.9 5 x 1/4" x 12.0	5 x 1/4" x 228.8 5 x 1/4" x 12.0

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A - L2X2X1/8



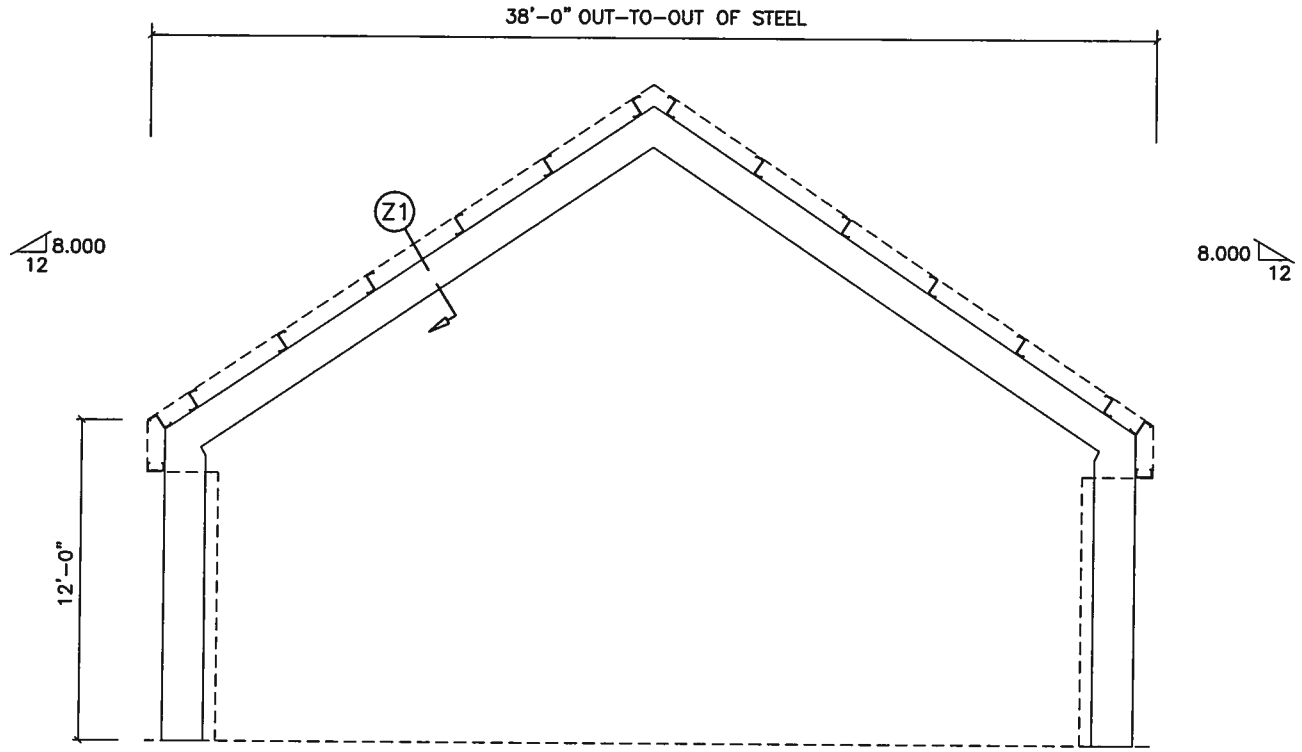
RIGID FRAME ELEVATION: FRAME LINE J

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

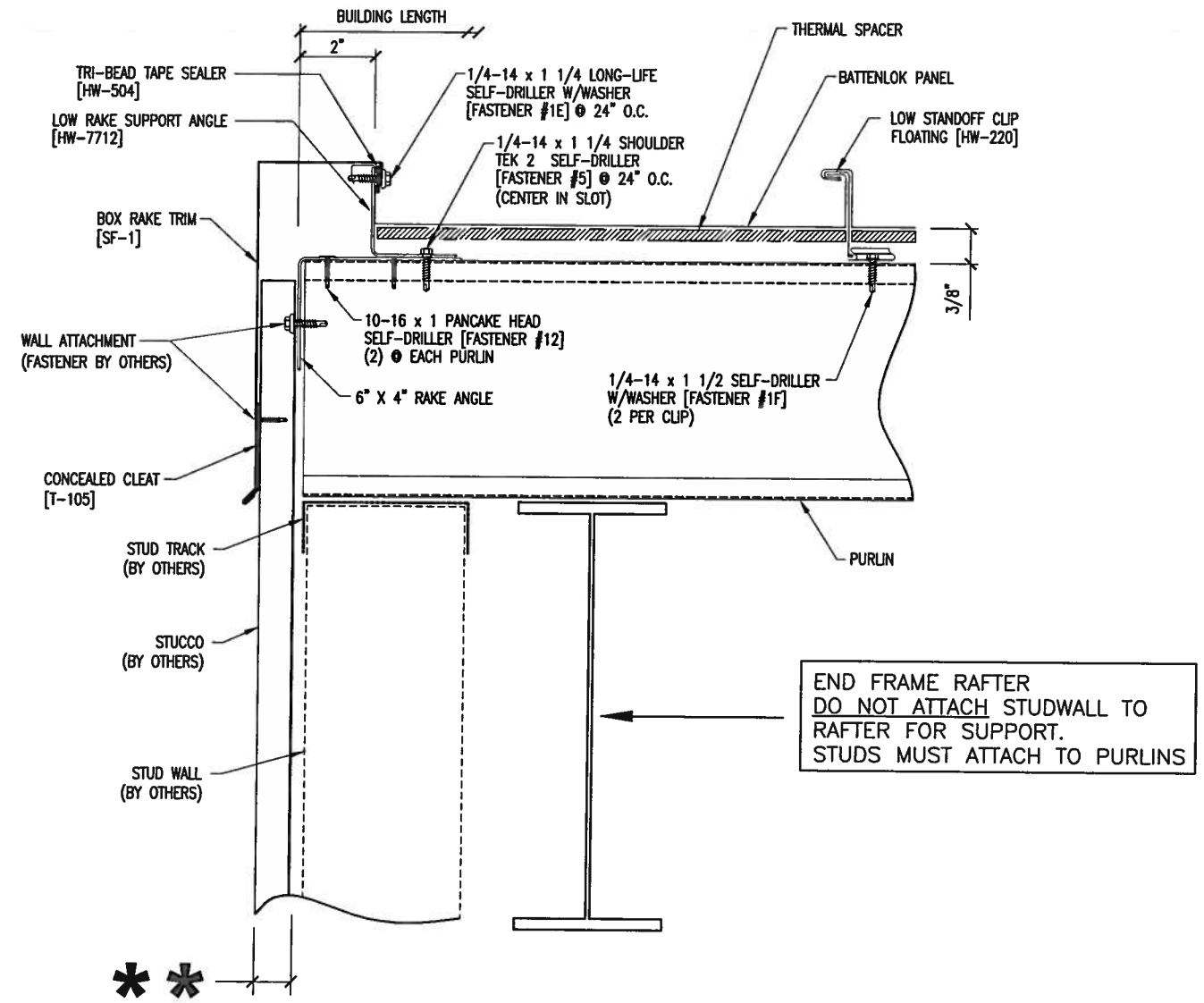
DESCRIPTION:	RIGID FRAME ELEVATION
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E06	Issue P



ENDWALL FRAMING: FRAME LINE 1 & 7

TYPICAL PANEL START DIMENSION IS 0'-2"



Z1 SECTION THROUGH ENDWALL

** PLEASE VERIFY WALL THICKNESS **

FOR PERMITS ONLY

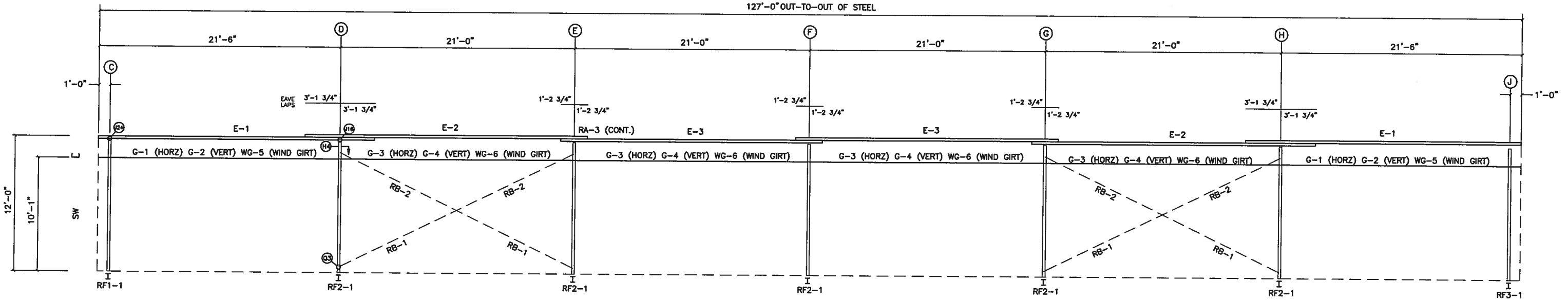
GENERAL NOTES:
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

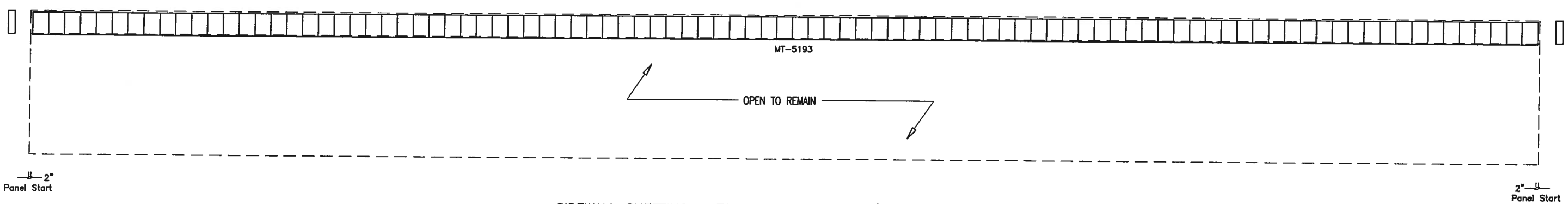
PINNACLE STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ENDWALL FRAMING
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E07	Issue P



SIDEWALL FRAMING: FRAME LINE 02 (FRAME LINE 01 OPP.)



SIDEWALL SHEETING & TRIM: FRAME LINE 02 (FRAME LINE 01 OPP.)

PANELS: 24 Ga. B-LOK

FOR PERMITS ONLY

GENERAL NOTES:

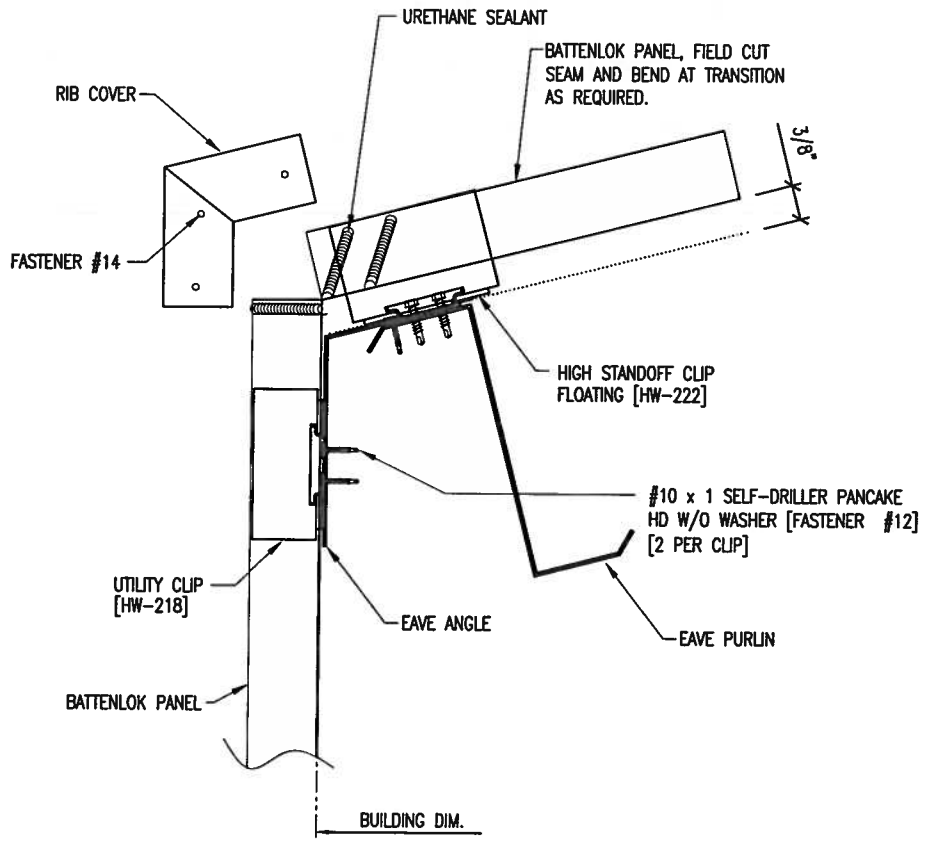
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

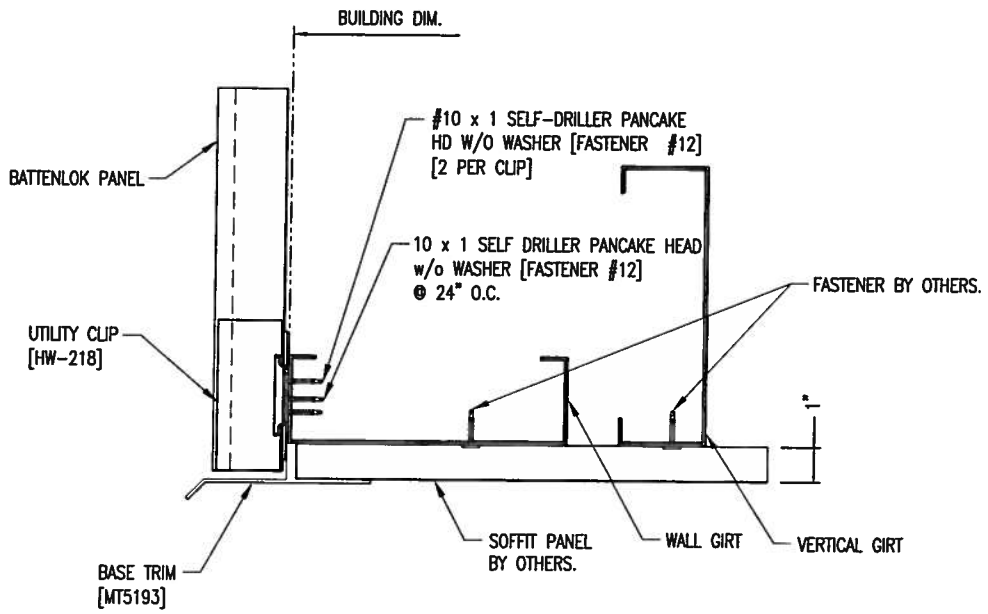
DESCRIPTION:	SIDEWALL FRAMING
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E08	Issue P



SECTION THROUGH ROOF TO WALL TRANSITION

SNOW RETENTION DEVICES, WHERE APPLICABLE, ARE BY OTHERS.



SECTION THROUGH WALL TO SOFFIT TRANSITION

FOR PERMITS ONLY

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	7/15/15	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	WALL SHEETING DETAILS
CUSTOMER:	7B BUILDING & DEVELOPMENT
LOCATION:	SAN ANGELO, TX

Detailer	Checker	Designer
JWF		
Job No. 151400A	Sheet E09	Issue P