

TRANSMITTAL

2665 Bill Foster Memorial Hwy
PO BOX 1268
CABOT, AR. 72023
PHONE: 501-941-3929
FAX: 501-941-2675

TO: **7B BUILDING & DEVELOPMENT**
13105 CR 1820
LUBBOCK, TX 79424

DATE: **11/15/2016**
JOB #: **161645A**

CONTACT: TYLER HOUCK

WE ARE SENDING YOU THE FOLLOWING ITEMS:

3 SETS OF ANCHOR BOLT PLANS SEALED
 WITH CERT. SHEET

SET OF REVISED ANCHOR BOLT PLANS SEALED
 WITH CERT. SHEET

1 SETS OF PERMIT DRAWINGS NOT FOR CONSTRUCTION SEALED
 WITH CERT. SHEET

SETS OF APPROVAL DWGS NOT FOR CONSTRUCTION SEALED
 WITH CERT. SHEET

Roof Panel Type: SSR PBR
(If SSR is checked, an installation manual will be provided.)

SETS OF FINAL DRAWINGS FOR CONSTRUCTION SEALED
 WITH CERT. SHEET

SETS OF ENGINEERING CALCULATIONS SEALED

SET OF: _____

EMAIL DWGS TO: **tyler@7bdev.com**

CC: **Josh and Don**

Notes: **Drawings for Champion Express Car Wash Tunnel Building 'A'**

PINNACLE STRUCTURES, INC
BY: **Donald Sale**

US MAIL 2ND DAY OVERNIGHT CUSTOMER PICK-UP



P. O. Box 1268
Cabot, Arkansas 72023
501-941-3929



Customer: 7B Buildings & Development
13105 CR 1820
Lubbock, TX 79424

Pinnacle Job #: 161645
Project: Champion Express
Project Location: Roswell, NM (Chaves Co.)
Project Description: 38'-0" W x 127' L x 12'-0" EH-8:12 Bldg A
31'-1.5" W x 21' L x 12'-0" EH - 8:12 Bldg B

This is to certify that the above referenced building or buildings has been designed in accord with the American Institute of Steel Construction (AISC-14th edition) and the 2010 AISC Seismic Provisions, American Iron and Steel Institute (AISI S100-2007) specifications and the IBC-2012 building code. The building and its components are designed and produced in an IAS approved facility by an IAS AC472 accredited manufacturer. Pinnacle Structures, Inc. is an IAS AC472 accredited manufacturer. Members are designed for the following loads specified on your Purchase Order:

Design Loads:

IBC Building Risk Category: II - Standard Risk

- 1) 2.2 PSF Metal Building Roof Dead Load
- 2) 5* PSF Collateral Load *
- 3) 20 PSF Secondary Live Load
- 4) 20 PSF Primary Live Load (Reducible per Code)
- 5) 40 PSF Ground Snow Load
- 6) 22.16 PSF Roof Snow Load
C_s = 1.0 C_t = 1.2 C_e = 0.91 I_s = 1.0
- 7) 115 MPH Wind Speed / V_{ult}¹ (10 Year Serviceability)
89 MPH Nominal Speed / V_{ult} = (V_{ult}* v0.6)

- 9) Crane: Quantity:
Type: N/A
Capacity: N/A Ton
Max. wheel load: N/A Kips
- 10) Mezzanine:
N/A
Dead Load: N/A PSF
Live Load: N/A PSF
- 11) Other:
N/A

- 8) Seismic: I_E = 1.0 Equivalent lateral-force procedure
S_S = 0.09 S_{DS} = 0.09
S₁ = 0.03 S₀₁ = 0.06
Site Class: D
Seismic Design Category: A

TRANSVERSE DIRECTION (Moment Frames)

Ordinary Steel Moment Frames (R=3.5, Q_s=3.0, Cd=3.0****)

END WALLS

Ordinary Steel Moment Frames (R=3.5, Q_s=3.0, Cd=3.0****)
Ordinary Steel Moment Frames (R=3.5, Q_s=3.0, Cd=3.0****)

Left Endwall: Non-Expandable Frame
Right Endwall: Non-Expandable Frame

LONGITUDINAL DIRECTION

Ordinary Steel Moment Frames (R=3.5, Q_s=3.0, Cd=3.0****)
Ordinary Steel Concentrically Braced Frames (R=3.25, Q_s=2, Cd=3.25****)

Sidewall: Portal Frame (where applicable)
Sidewall: Rod Bracing (where applicable)

- ¹Note: This project is designed as an Partially Enclosed (GCp=±0.65) Building. Accessories (doors, windows, etc.) by others must be designed as "components and cladding" per the referenced Building Code.
- *Note: This project is designed for this collateral loading. NO additional loads shall be attached to the Pinnacle structure that will exceed this loading. All loads suspended from the purlins must be attached to the purlin webs and not the purlin flanges. Under NO circumstances are the purlin flanges to be modified by cutting, drilling or bending of flange or lip.
- ****Note: The shown values of C_d were used in determination of PΔ effects. Per ASCE7-10 Sec. 12.12.1 (Table 12.12-1) , this structure is designed for story drift requirements. Any interior ceilings, partitions, walls and other attached elements must be detailed to accommodate story drifts. The seismic drift limit used is H/50.

This Letter of Certification applies solely to the structural framing and its component parts as furnished by Pinnacle Structures, Inc. and specifically excludes any foundation, masonry, or general contract work to include erection certification.

The design and Certification for this project is in accord with the provisions and loads specified on the building order. The buyer is responsible to verify specified loads are in compliance with the local regulatory authorities.

The undersigned is not the engineer of record for the overall project.

Sincerely,



11-22-16

GENERAL NOTES

1. This structure has been designed in accordance with the 2007 AISI NAUS Cold Formed Steel Design Manual and the AISI (14th Edition, ASD) Steel Construction Manual.

2. Fabrication shall be accordance with Pinnacle Standards in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1".

Materials	ASTM Designation	Minimum Yield
Hot Rolled Angle	A36	Fy = 36 ksi
Structural Steel Plate	A572, A529, A1011	Fy = 55 ksi
Cold Formed Shapes	A1011/(A653 Galvanized)	Fy = 55 ksi
Cable Bracing	A475	Ex. High Strength
Roof & Wall Sheeting	A792 26 GA A792 24 and 22 GA	80 ksi, Class 1 50 ksi, Class 2
High Strength Bolts	A325-Group A/(A490-Group B)	
Pipe	A53, Gr. B	Fy = 35 ksi
Round Structural Tubing	A500, Gr. B	Fy = 42 ksi
Shaped Structural Tubing	A500, Gr. B	Fy = 46 ksi
Hot Rolled Shapes	A572, A992, A529 Gr. 50	Fy = 50 ksi
Hot Rolled Shapes	A36	Fy = 36 ksi

4. Shop primer paint is a rust inhibitive primer which meets the end performance of SSPC-Paint 15: Steel Joist Shop Primer/ Metal Building Primer and is maroon oxide in color. This paint is not intended for long term exposure to the elements. Pinnacle Structures, Inc. is not responsible for any deterioration of the shop primer as a result of improper handling or storage. Pinnacle will not be responsible for any field applied paint and/or coatings. (Section 7.17 AISI code of Standard Practice for Steel Buildings & Bridges, 13th Edition).

5. Bolts for the construction of Pinnacle Structures, Inc. material shall be as follows:
 All secondary member connections - 1/2" x 1 1/4" A307 unless noted
 Bearing frame endwall connections - A325
 Main frame connections - A325 as shown on drawings

6. Connections Using High Strength Structural Bolts:
 All high strength bolts are A325-N, unless noted otherwise. High strength structural bolts are supplied without washers, unless noted otherwise. Bolt length shall be such that the end of the bolt extends beyond or is at least flush with the outer face of the nut, when properly installed. All bolted connections, unless noted, are designed as bearing type connection with bolt threads not excluded from the shear plane.

A325-N High Strength Structural Bolts:
 Snug-Tightened connections are permitted with A325-N bolts, except for these cases:
 - Where crane beams and rigid frame connections in crane buildings are present
 - In Slip-Critical Connections
 - If noted in the erection drawings otherwise
 For these exceptions, Turn-of-the-Nut method must be used.

A490 High Strength Structural Bolts:
 A490 structural bolts shall be tightened using the Turn-of-the-Nut method. Snug-Tightened connections are not permitted with A490 bolts.

Tightening Methods:

Snug-Tightened Joint: A condition in which the tightness that exists when all of the plies in a connection have been pulled into firm contact by the bolts in the joint and all of the bolts in the joint have been tightened sufficiently to prevent the removal of the nuts without the use of a wrench, in accordance with the 14th Edition of AISI "Specification for Structural Joints Using High-Strength Bolts", per Section 8.1.

Turn-of-the-Nut method is to be performed in accordance with the 14th Edition AISI "Specification for Structural Joints Using High-Strength Bolts" per Section 8.2.1.

7. All Bracing shown and provided by Pinnacle for this building is required for transferring building loads to the foundation and shall be installed by the erector as a permanent part of the structure. Cable/Rod bracing is designed for structural loads only and is not designed to plumb the building. The cable/rod bracing shall be taut, tighten to remove sag only. Bracing shall not be over-tighten. If additional bracing is required for stability during erection, it shall be the erectors responsibility to determine the amount of such bracing and to procure and install as necessary.

8. Soil profile type is determined by the foundation Engineer per local code.

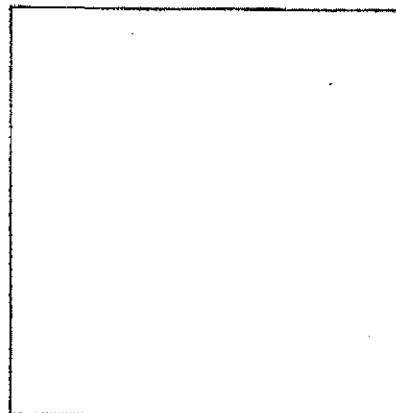
9. Building Codes Require Consideration of Snow Surcharges for Any Lower Roof of a Structure Located within 20 Feet of a Higher Structure. Information Supplied to Pinnacle Structures Does Not Indicate the Presence of a Shadowing Structure within this 20 Foot Envelope. Therefore Snow Surcharges Have Not Been Considered in this Design Unless Noted Otherwise.



2665 Bill Foster Mem. Hwy, P.O. Box 1268
 Cabot, AR 72023
 Phone: (501) 941-3929 or (800) 201-1534
 Fax: (501) 941-2675



AC472 ACCREDITED
 MB-103



DRAWING PACKAGE FOR:

JOB NUMBER: 161645A
 CUSTOMER: 7B Building & Development
 PROJECT: Champion Express
 JOBSITE: Roswell, NM (Chaves Co.)
 BUILDING SIZE: 38' x 127' x 12' (8:12)

DESIGN REQUIREMENTS

Building Code: IBC 2012
 IBC Building Risk Category: II - Standard Risk
 Metal Building Dead Load: 2.2 psf
 Collateral Load: 5 * psf
 Secondary Live Load: 20 psf
 Primary Live Load: 20 psf (REDUCIBLE PER CODE)
 Ground Snow Load: 40 psf
 Roof Snow Load: 22.18 psf
 Ce= 1.0 Ct= 1.2 Cs= 0.91 Is = 1.0
 115 MPH Wind Speed / Vult¹ (10 Year Serviceability)
 89 MPH Nominal Speed / V_{nom} = (Vult* v-0.6)
 Exposure = C
Seismic: EQUIVALENT LATERAL FORCE PROCEDURE
 Seismic Coefficient: I_e= 1.0 S_s= 0.09 S_{DS}= 0.09
 S₁= 0.03 S_{D1}= 0.08
 Site Class: D
 Seismic Design Category: A

TRANSVERSE DIRECTION (Moment Frames)

Ordinary Steel Moment Frames (R=3.5, Ω₀=3.0, Cd=3.0***)

END WALLS

Ordinary Steel Moment Frames (R=3.5, Ω₀=3.0, Cd=3.0***)

Ordinary Steel Moment Frames (R=3.5, Ω₀=3.0, Cd=3.0***)

Left Endwall: Non-Expandable Rigid Frame
 Right Endwall: Non-Expandable Rigid Frame

LONGITUDINAL DIRECTION

Ordinary Steel Moment Frames (R=3.5, Ω₀=3.0, Cd=3.0***)

Ordinary Steel Concentrically Braced Frames (R=3.25, Ω₀=2.0, Cd=3.25***)

Front Sidewall: Portal Frame (where applicable)
 Back Sidewall: Rod Bracing (where applicable)

Crane:

Type: N/A
 Capacity: N/A Ton
 Max. wheel load
 not including impact N/A Kips

Mezzanine:

Dead Load: N/A psf
 Live Load: N/A psf

Snow Drift:

Pd: N/A psf
 Wd: N/A feet

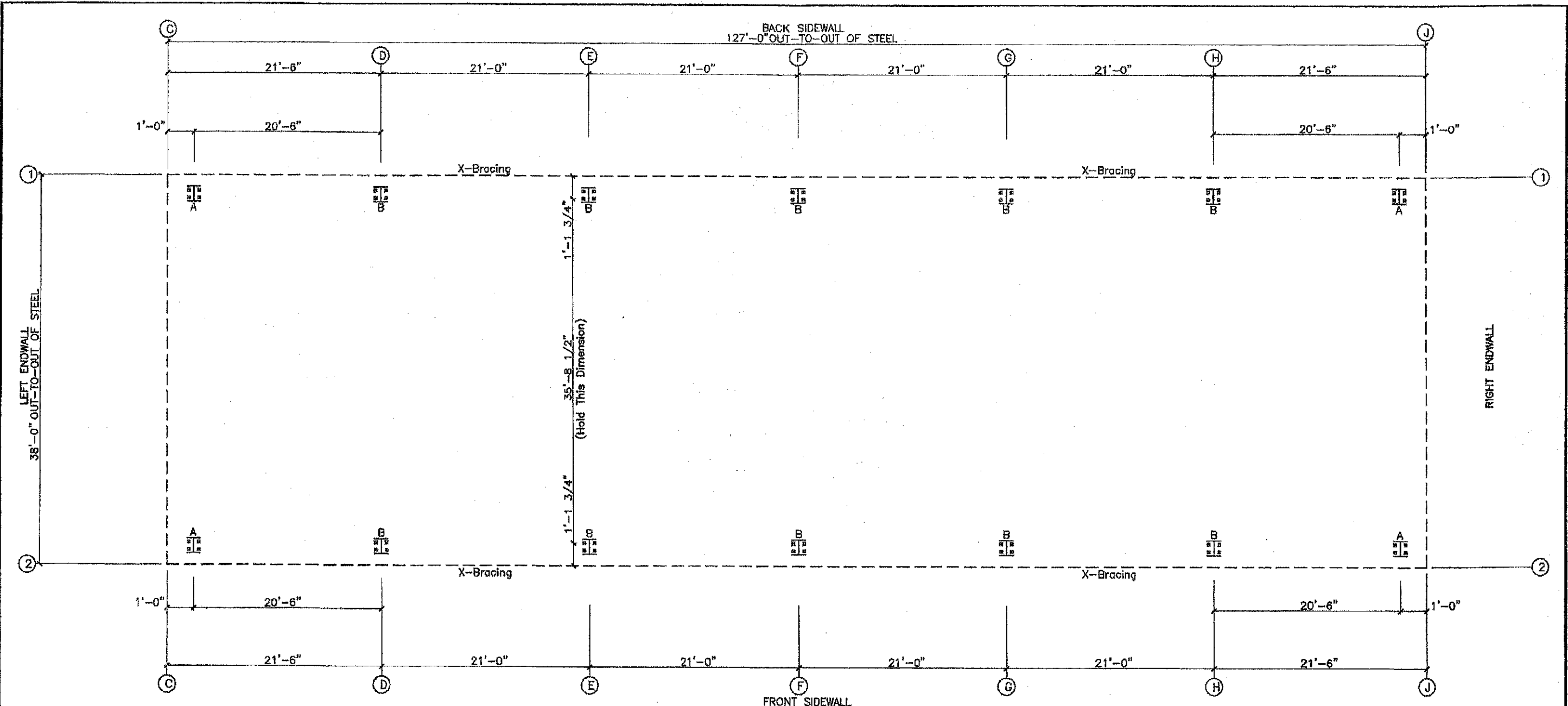
Other: N/A



11-22-16

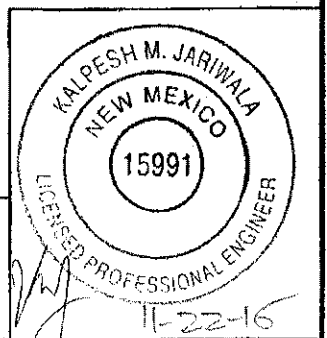
* This project is designed for this collateral loading. NO additional loads shall be attached to the Pinnacle structure that will exceed this loading. All loads suspended from the purlins must be attached to the purlin webs and not the purlin flanges. Under NO circumstances are the purlin flanges to be modified by cutting, drilling or bending of flange or lip.

*/***/ SEE LETTER OF CERTIFICATION.

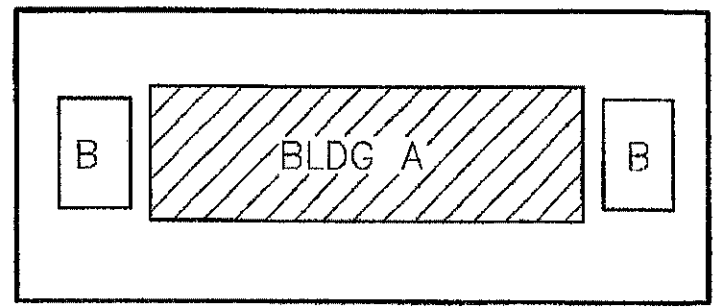
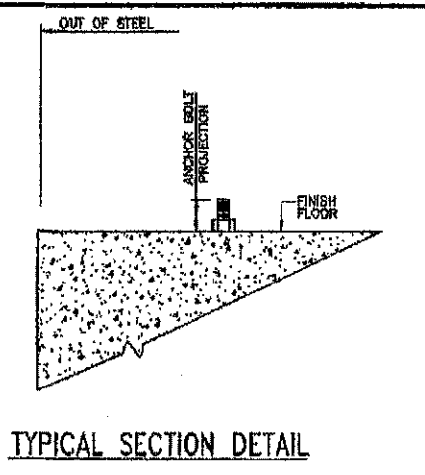


ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0"

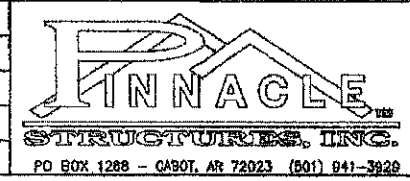
ANCHOR BOLT SUMMARY				
Qty	Locate	Dia (in)	Type	Pral (in)
56	Frame	3/4"	A307	2.50



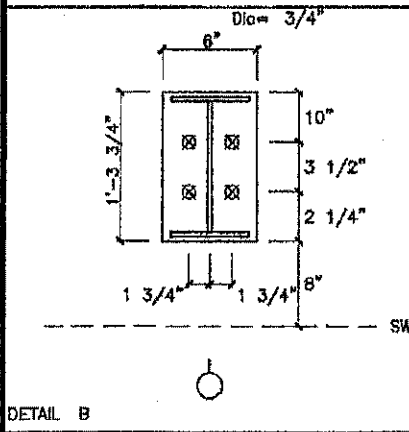
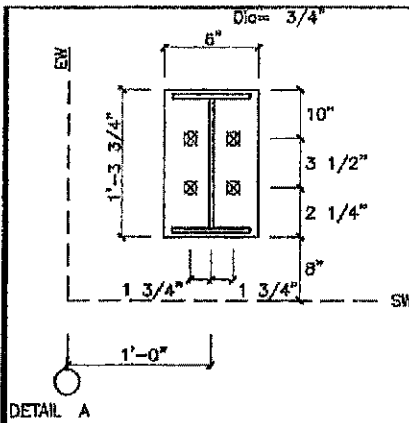
- GENERAL NOTES**
- These drawings are NOT to scale.
 - Pinnacle's steel line is shown.
 - A sheeting notch or brick ledge, if used, must be added to determine the out of concrete.
 - Wall panels shall be held 1/4" above the sheet steel and/or base trim.
 - Attachment of material by others to Pinnacle steel is the responsibility of others.



ISSUE	DESCRIPTION	DATE	MARK
0	CONSTRUCTION	11/14/16	



DESCRIPTION:	ANCHOR BOLT PLAN		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer	SS	Checker	DS
Designer	SO		
Job No.	181645A	Sheet	F1
Issue	0		



NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 38.0
 - Length (ft) = 127.0
 - Eave Height (ft) = 12.0 / 12.0
 - Roof Slope (rise/12) = 8.0 / 8.0
 - Dead Load (psf) = 2.2
 - Collateral Load (psf) = 5.0
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 12.0
 - Snow Load (psf) = 2.2
 - Wind Speed (mph) = 115.0
 - Wind Code = ASCE 12
 - Exposure = C
 - Closed/Open = P
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = A
 - Seismic Coeff (Fa*Sa) = 0.15

5. Loading conditions are:
- Dead+Collateral+0.75Snow+0.45Wind_Left1
 - Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Slide_Snow
 - Dead+Collateral+0.75Snow+0.45Wind_Right1+0.75Slide_Snow
 - Dead+Collateral+0.75Snow+0.45Wind_Left2+0.75Slide_Snow
 - Dead+Collateral+0.75Snow+0.45Wind_Right2+0.75Slide_Snow
 - 0.8Dead+0.8Wind_Left2
 - 0.8Dead+0.8Wind_Right2
 - 0.8Dead+0.8Wind_Long1
 - 0.8Dead+0.8Wind_Long2

BUILDING BRACING REACTIONS

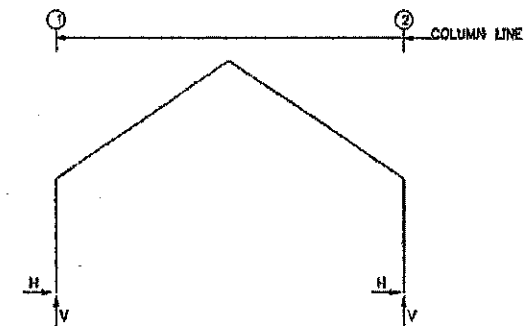
Wall Loc	Col Line	± Reactions (k)	Wind		Seismic		Panel Shear (lb/ft)	Note
			Horz	Vert	Horz	Vert		
L_EW	C	1.8	0.9	0.8	0.4			(h)
F_SW	2	1.8	0.8	0.8	0.4			(h)
R_EW	J	1.8	0.9	0.8	0.4			(h)
B_SW	1	1.8	0.8	0.8	0.4			(h)

(h) Rigid frame at endwall

GENERAL NOTES

- ANCHOR BOLTS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
- FOUNDATION DESIGN AND ANCHOR BOLT LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR BOLT SUMMARY TABLE REPORTS THE BOLT DIAMETERS.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8.

FRAME LINES: C D E F G H J



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load ID	Column Reactions (k)				Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in)			Grout (in)	
			Hmax	Vmax	Hmin	Vmin			Width	Length	Thick		
C*	1	3	3.7	5.0	8	-2.6	1.6	4	0.750	6.000	15.75	0.375	0.0
C*	2	7	5.2	7.9	9	0.3	-3.2	4	0.750	6.000	15.75	0.375	0.0

C* Frame lines: C J

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load ID	Column Reactions (k)				Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in)			Grout (in)	
			Hmax	Vmax	Hmin	Vmin			Width	Length	Thick		
D*	1	3	6.0	8.5	8	-3.8	2.8	4	0.750	6.000	15.75	0.375	0.0
D*	2	7	3.6	2.8	2	-6.0	8.5	4	0.750	6.000	15.75	0.375	0.0

D* Frame lines: D E F G H

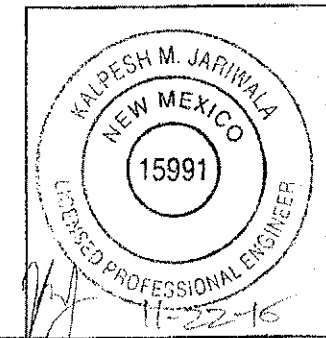
RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind Left		Wind Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
D*	1	0.5	1.6	0.8	2.6	1.8	4.8	3.0	8.1	-5.0	-8.4	5.5	-5.3
D*	2	-0.5	1.6	-0.8	2.6	-1.6	4.8	-3.0	8.1	-5.0	-8.4	5.0	-5.4
C*	1	0.3	1.1	0.5	1.4	1.0	2.8	1.7	5.1	-3.8	-5.0	3.8	-2.9
C*	2	-0.3	1.1	-0.5	1.4	-1.0	2.8	-1.7	5.1	-3.8	-5.0	3.8	-5.0

Frame Line	Column Line	Wind Left2		Wind Right2		Wind Long1		Wind Long2		Seismic Left		Seismic Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
D*	1	-5.9	3.0	3.7	8.1	2.6	-11.8	0.3	-12.3	-0.2	-0.1	0.2	0.1
D*	2	-3.7	6.1	6.9	3.0	-0.3	-12.3	-2.8	-11.8	-0.2	0.1	0.2	-0.1
C*	1	-4.6	1.3	2.7	3.6	1.4	-8.1	0.2	-6.4	-0.3	-0.2	0.3	0.2
C*	2	-2.7	3.5	4.6	1.3	-0.2	-6.4	-1.4	-6.1	-0.3	0.2	0.3	-0.2

Frame Line	Column Line	Seismic Long	
		Horz	Vert
D*	1	0.0	-0.4
D*	2	0.0	-0.4
C*	1	0.0	0.0
C*	2	0.0	0.0

D* Frame lines: D E F G H
C* Frame lines: C J



ISSUE	DESCRIPTION	DATE	MARK
0	CONSTRUCTION	11/14/16	

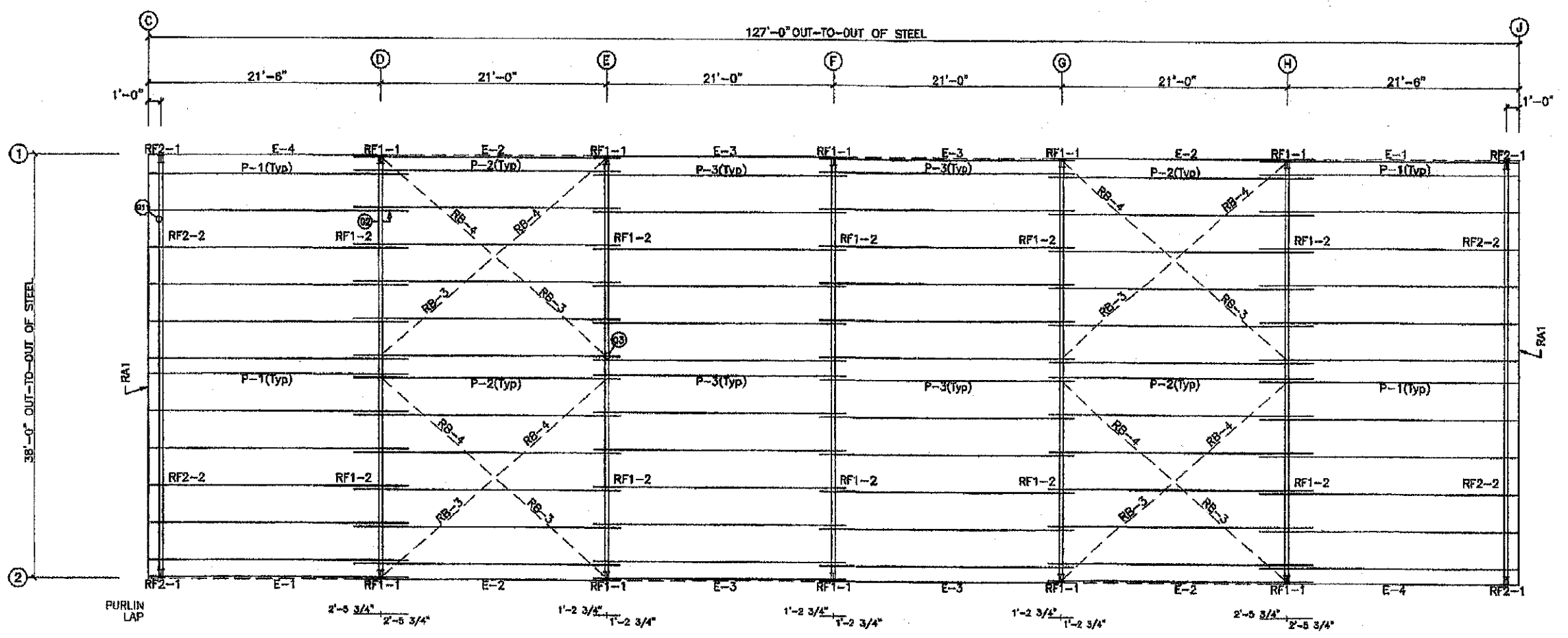


DESCRIPTION:	ANCHOR BOLT DETAILS & REACTIONS		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer:	SS	Checker:	DS
Designer:	So		
Job No.:	161645A	Sheet:	F2
Issue:	0		

PD BOX 1288 - CABOT, AR 72023 (501) 941-3828

MEMBER TABLE	
ROOF PLAN	
MARK	PART
P-1	8X25Z14
P-2	8X25Z16
P-3	8X25Z16
E-1	8X25Z16
E-2	8X25Z16
E-3	8X25Z16
E-4	8X25Z16
RB-3	1/2" ROD
RB-4	1/2" ROD

NOTE:
 Alternate Arrows V-Δ
 Up And Down From Bay
 To Bay For Purlins To Lap.



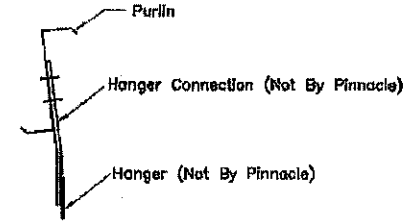
ROOF FRAMING PLAN



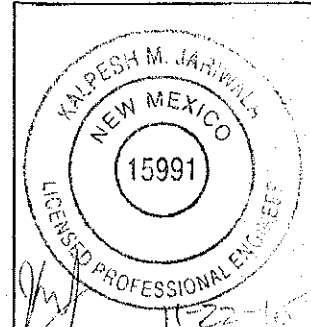
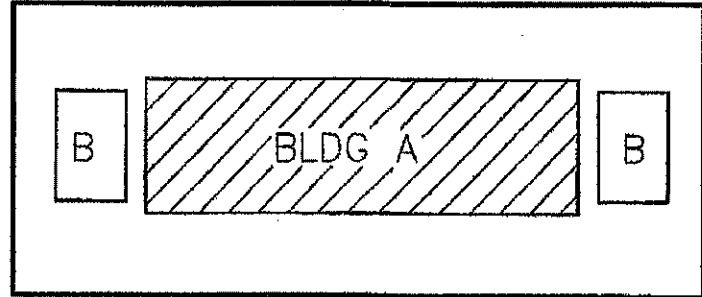
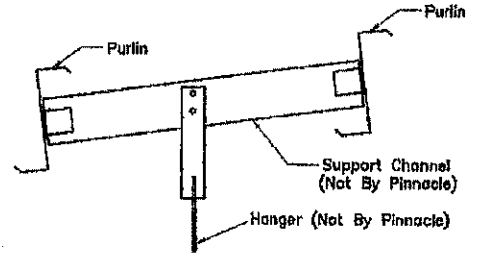
ROOF SHEETING
 PANELS: 24 Ga. PBR
 Brite Red

COLLATERAL LOAD NOTE:
 Roof purlin has been designed for the collateral load listed on the cover. The total applied loads due to ceiling panels, ducts, sprinkler distribution lines, electrical equipment, conduit, fireproofing, other piping or mechanical loads cannot exceed this maximum uniform load. Pinnacle Structures, Inc. is not responsible for lateral or longitudinal bracing of suspended members subject to lateral seismic or wind loading.

Loads supported directly from the purlins must have connections through the web of the purlin.



Loads supported between purlins must be supported such that the loads are applied to the webs of the purlins.

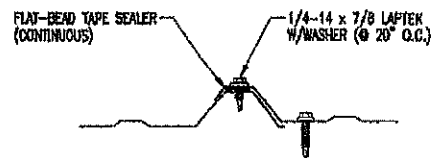


FOR PERMIT ONLY

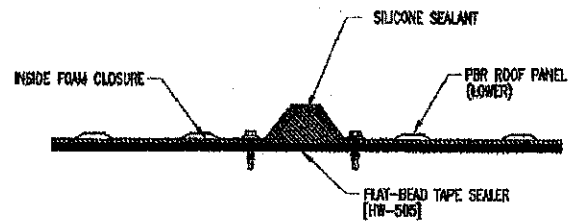
ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	11/14/16	



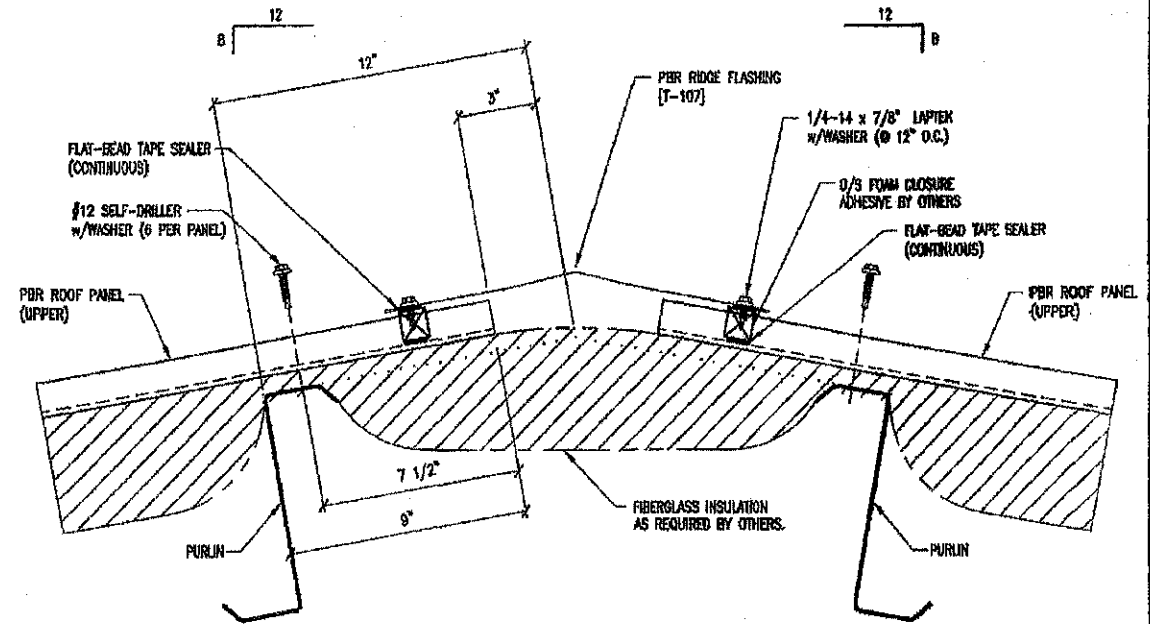
DESCRIPTION:	ROOF FRAMING		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer:	SS	Checker:	DS
Designer:	SO	Job No.:	161645A
Sheet:	E1	Issue:	P



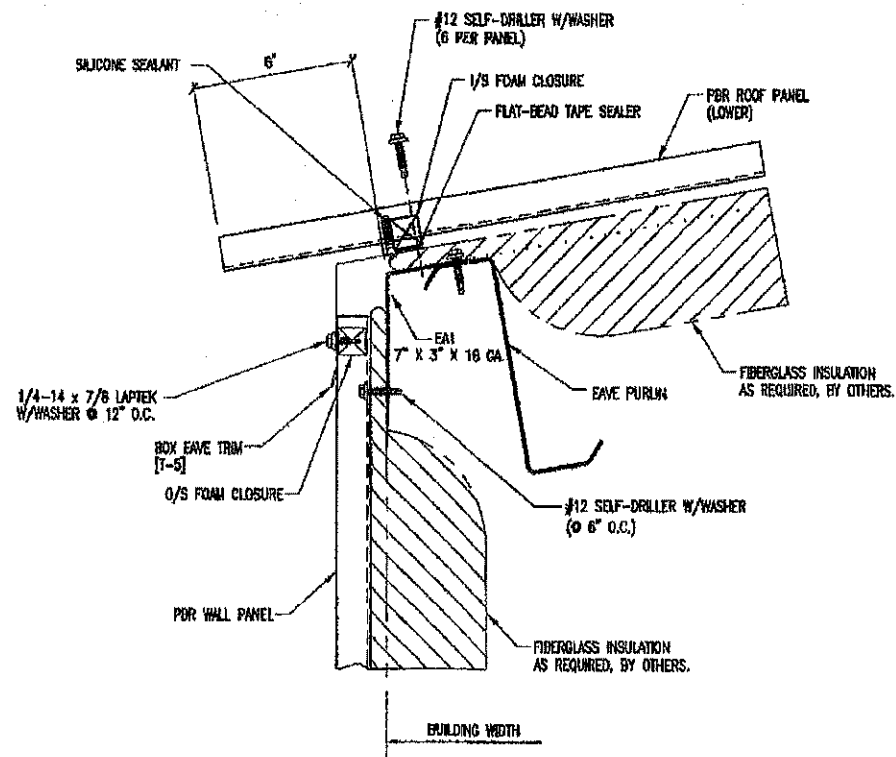
SECTION THROUGH PANEL SIDELAP



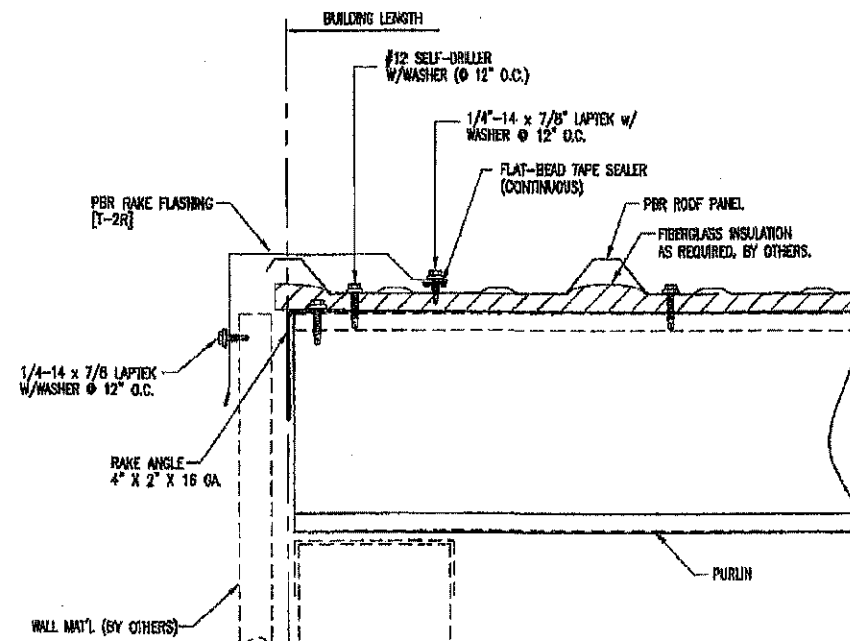
SECTION THROUGH EAVE END



SECTION THROUGH PBR RIDGE FLASHING
THIS DETAIL IS USED ON ROOF SLOPES OF 5:12 AND GREATER

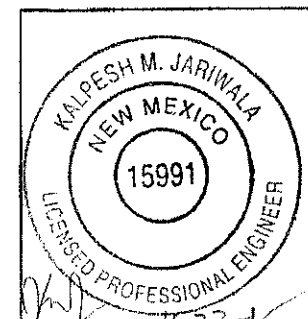


SECTION THROUGH EAVE PURLIN
WITHOUT GUTTER



SECTION THROUGH RAKE START
AVERAGE START DIMENSION IS 0'-0\"/>

FOR PERMITS ONLY



ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	11/14/16	

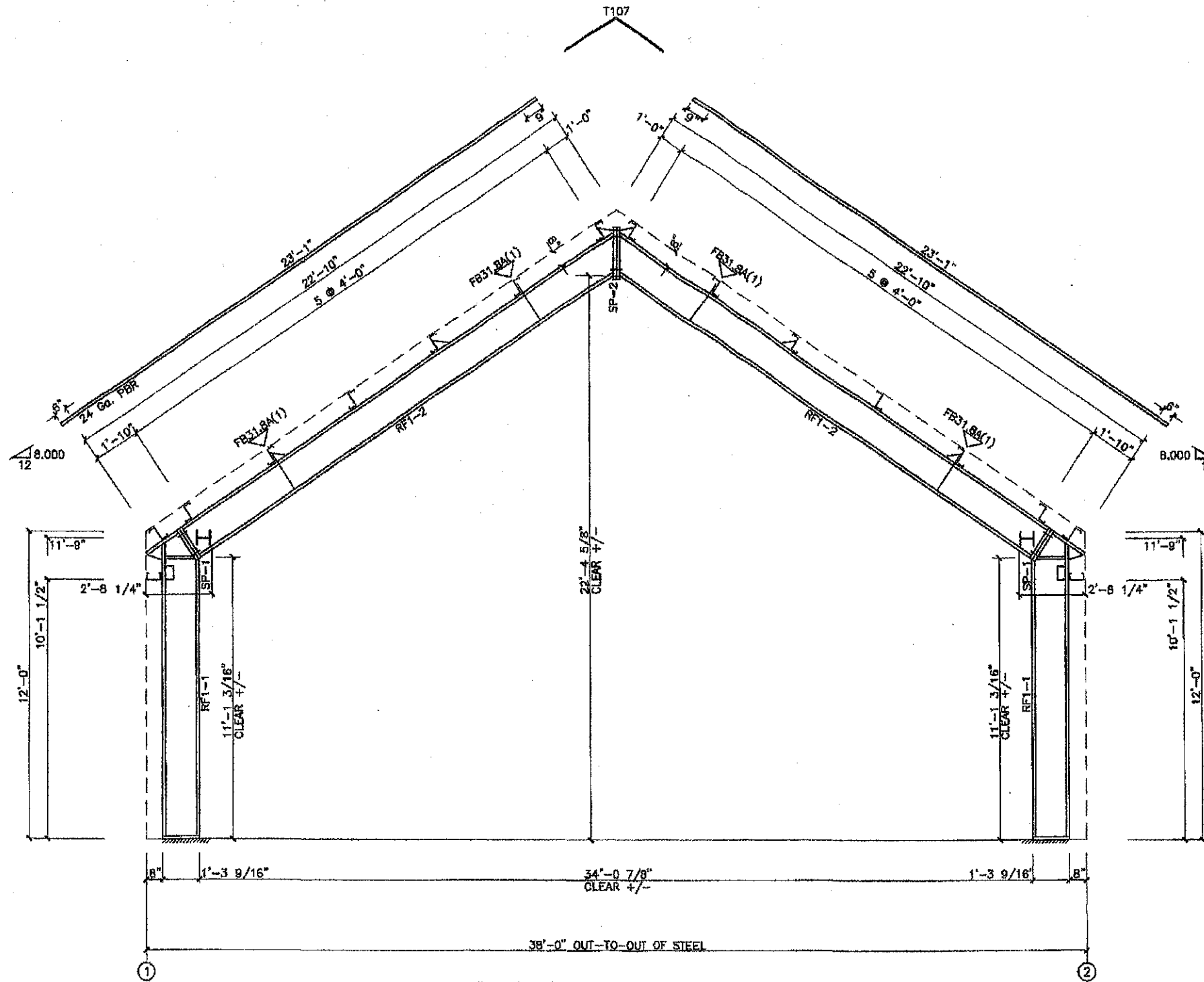


DESCRIPTION:	PBR ROOF PANEL DETAILS		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer	SS	Checker	DS
Job No.	161645A	Sheet	E2
Designer	SD	Issue	P

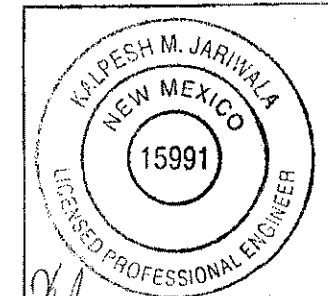
SPICE PLATE & BOLT TABLE										
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	2	2	0	0	A325	1"	2 3/4"	6"	3/4"	1'-2 5/8"
SP-2	4	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	2'-2"

MEMBER TABLE							
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length	W x Thk x Length
RF1-1	15.0	15.0	0.135	132.0	5 x 1/4" x 130.0	5 x 5/16" x 132.0	
RF1-2	15.0	7.1	0.250	11.9	5 x 1/4" x 18.2		
	14.0	14.0	0.135	240.0	5 x 1/4" x 240.0	5 x 5/16" x 232.1	
	14.0	14.0	0.135	13.7	5 x 1/4" x 13.7	5 x 5/16" x 12.0	

FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxxA(1): xx=length(in)
 A = L2K2x1/8



RIGID FRAME ELEVATION: FRAME LINE D E F G H



FOR PERMITS ONLY

11-22-16

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	11/14/16	

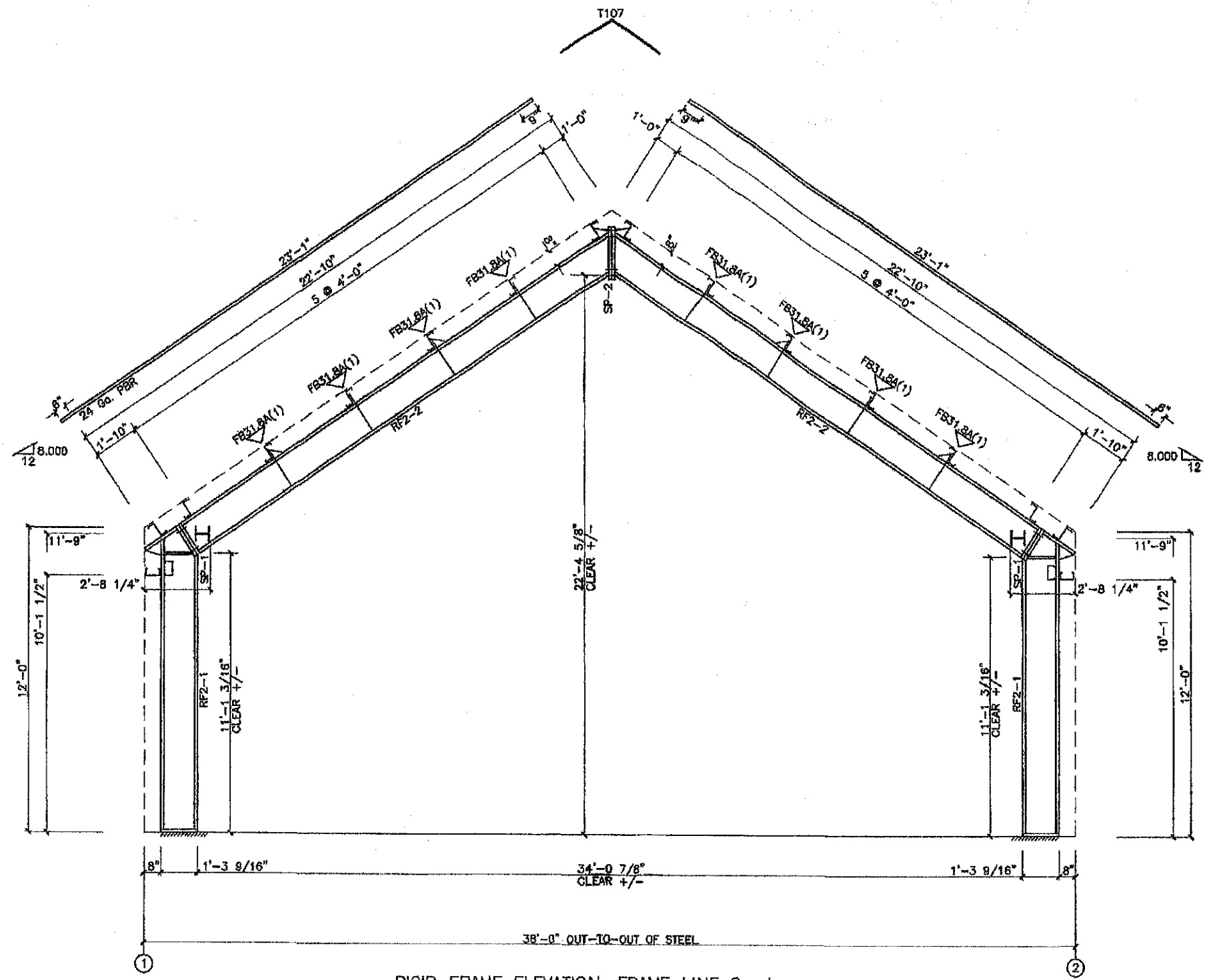


DESCRIPTION:	RIGID FRAME ELEVATION		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer	SS	Checker	DS
Designer	SO		
Job No.	161645A	Sheet	E3
Issue	P		

SPLICE PLATE & BOLT TABLE										
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	2	2	0	0	A325	3/4"	2 1/2"	6"	3/4"	1'-2 5/8"
SP-2	4	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	2'-2"

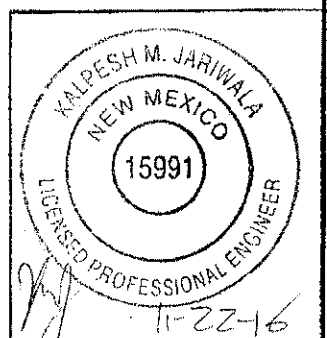
MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	
	Start	End	Thick	Length	W x Thk x Length	Inside Flange
RF2-1	15.0	15.0	0.135	143.9	5 x 1/4" x 138.0	5 x 5/16" x 132.0
RF2-2	14.0	14.0	0.135	240.0	5 x 1/4" x 240.0	5 x 5/16" x 232.1
	14.0	14.0	0.135	13.7	5 x 1/4" x 13.7	5 x 5/16" x 12.0

FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxA(1): xx=length(in)
 A - L2X2X1/8



RIGID FRAME ELEVATION: FRAME LINE C J

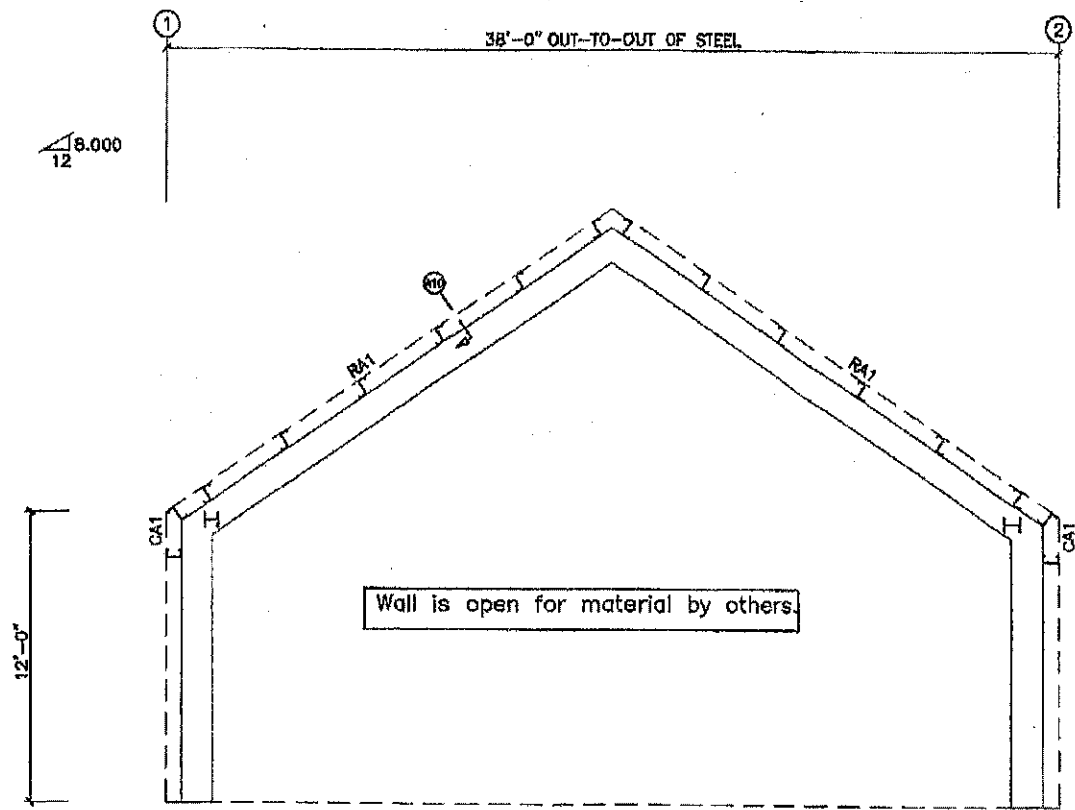
FOR PERMITS ONLY



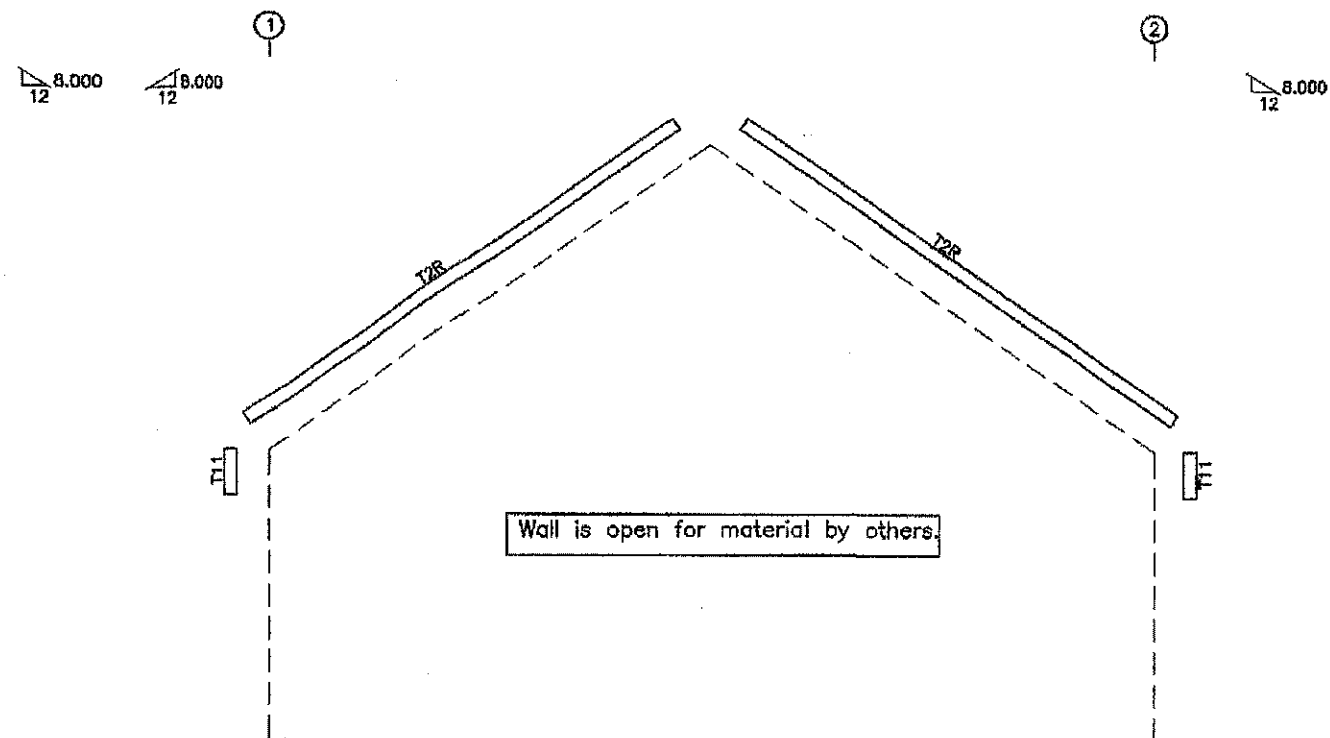
ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	11/14/16	

PO BOX 1258 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION: RIGID FRAME ELEVATION		
CUSTOMER: 7B BUILDING & DEVELOPMENT		
LOCATION: ROSWELL, NM (CHAVES CO.)		
Detailer: SS	Checker: DS	Designer: JS
Job No. 161645A	Sheet: E4	Issue: P



ENDWALL FRAMING: FRAME LINE C

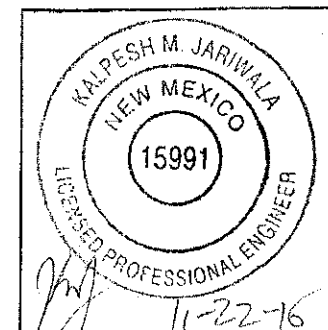


ENDWALL TRIM: FRAME LINE C

GENERAL NOTES:

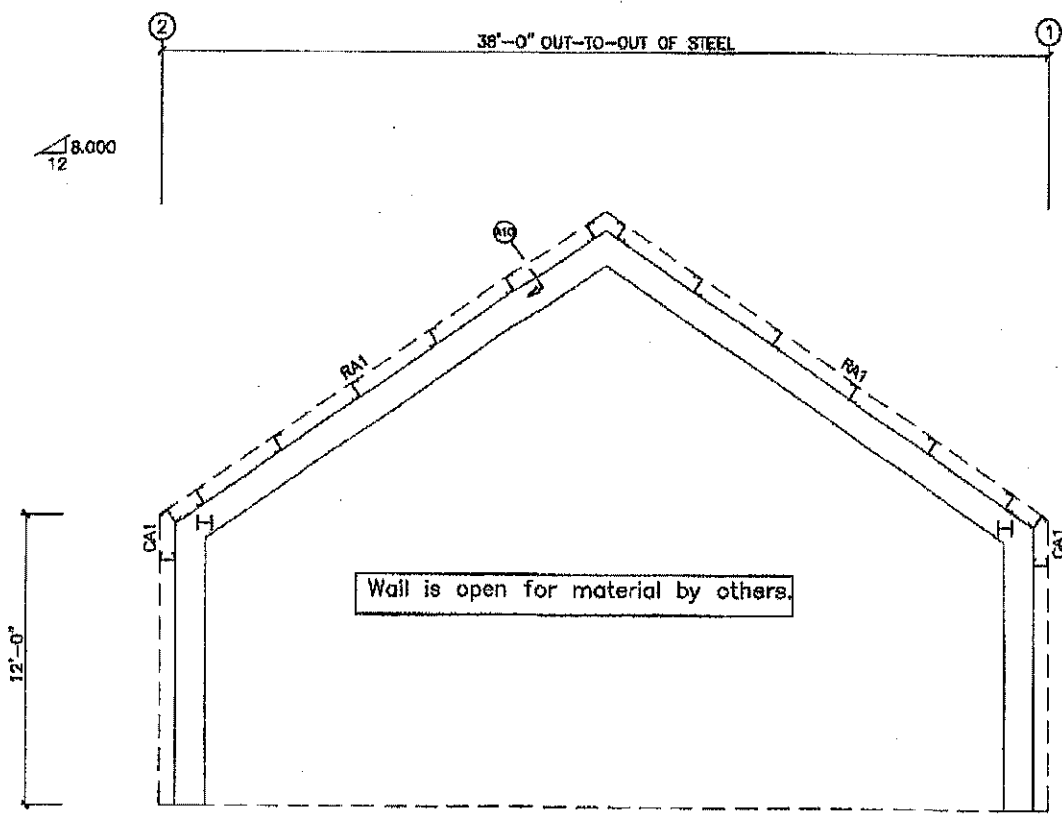
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.

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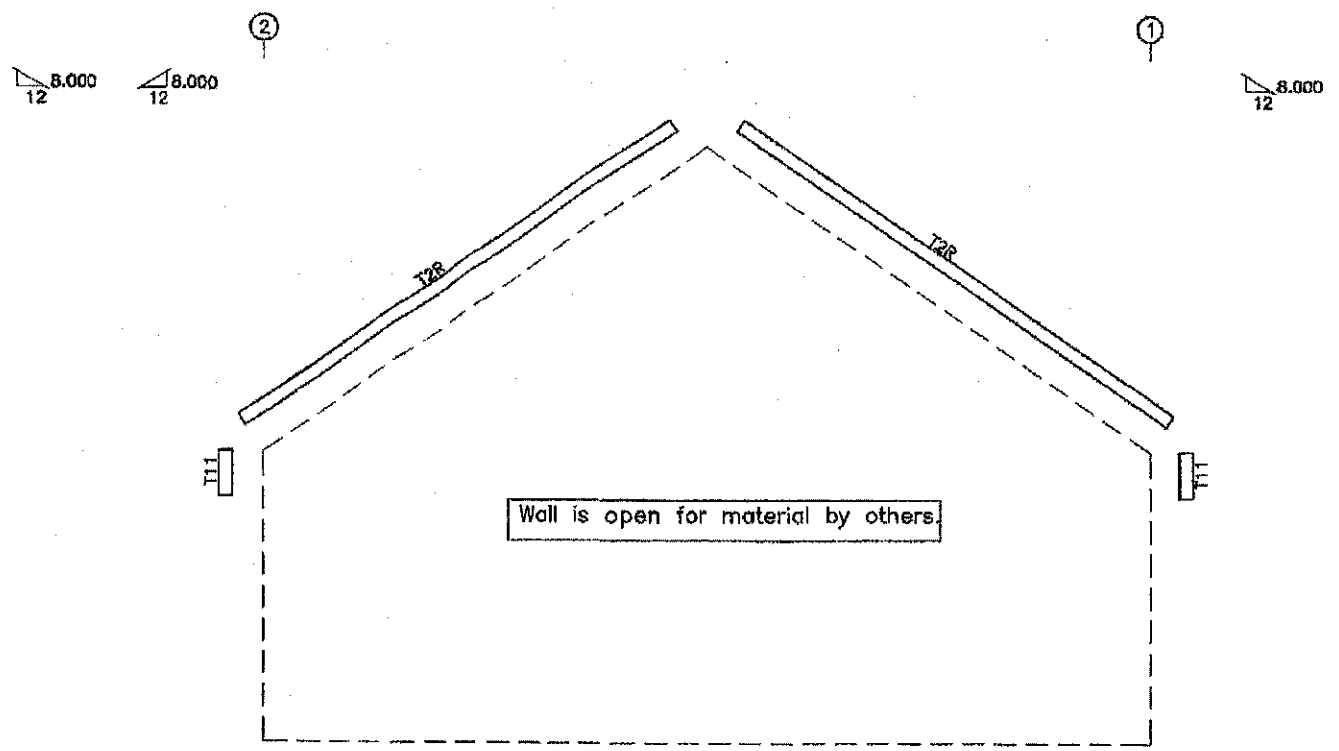


ISSUE	DESCRIPTION	DATE	MARK	DESCRIPTION: ENDWALL FRAMING		
P	PERMIT	11/14/16		CUSTOMER: 7B BUILDING & DEVELOPMENT		
				LOCATION: ROSWELL, NM (CHAVES CO.)		
				Detailer SS	Checker DS	Designer SO
				Job No. 161645A	Sheet E5	Issue P





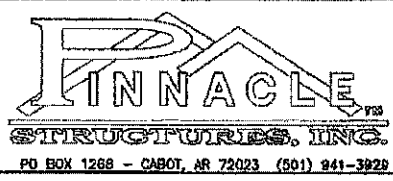
ENDWALL FRAMING: FRAME LINE J



ENDWALL TRIM: FRAME LINE J

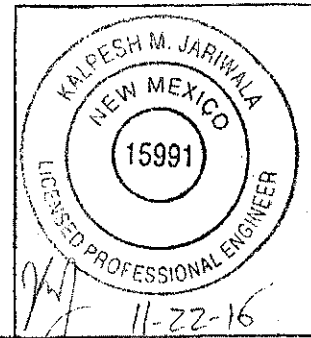
GENERAL NOTES:
 1. Pinnacle standard trim lap is 3 inches max.
 2. Pinnacle pre-cuts wall panels at factory located openings as required.
 3. Slot girts in field for cable passage at flush walls as required.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	11/14/18	

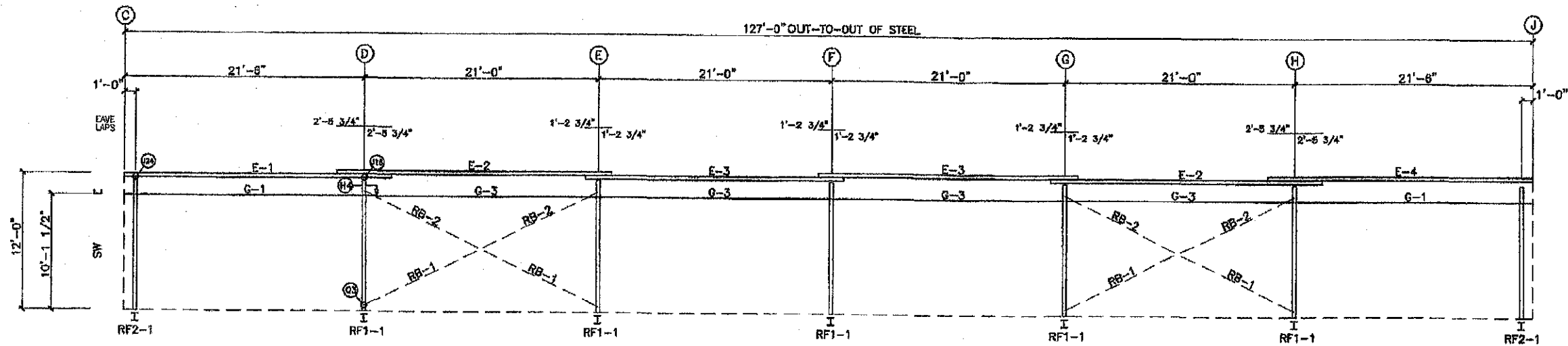


DESCRIPTION:	ENDWALL FRAMING		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer	SS	Checker	DS
Designer	SD		
Job No.	161645A	Sheet	EB
Issue	P		

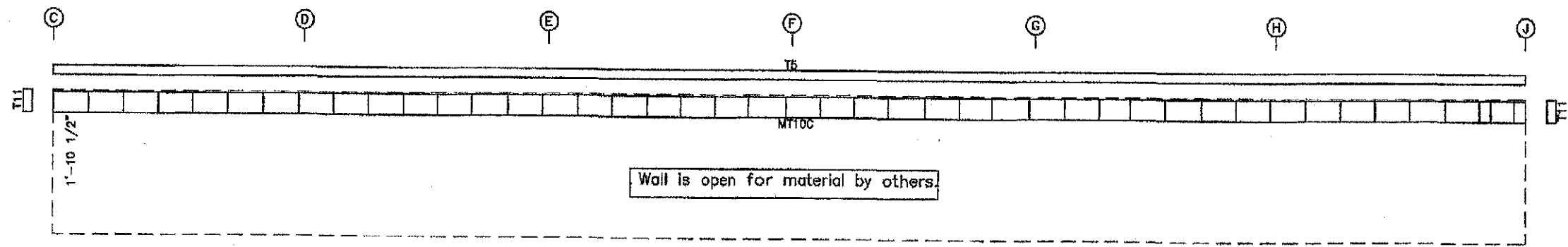
FOR PERMITS ONLY



MEMBER TABLE	
FRAME LINE 2	
MARK	PART
E-1	8X25Z16
E-2	8X25Z16
E-3	8X25Z16
E-4	8X25Z16
G-1	8X25C16
G-2	W08541
G-3	8X25C16
G-4	W08541
RB-1	1/2" ROD
RB-2	1/2" ROD

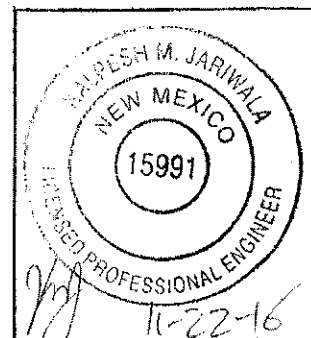


SIDEWALL FRAMING: FRAME LINE 2



SIDEWALL SHEETING & TRIM: FRAME LINE 2
PANELS: 24 Ga. PBR - Brite Red

FOR PERMITS ONLY



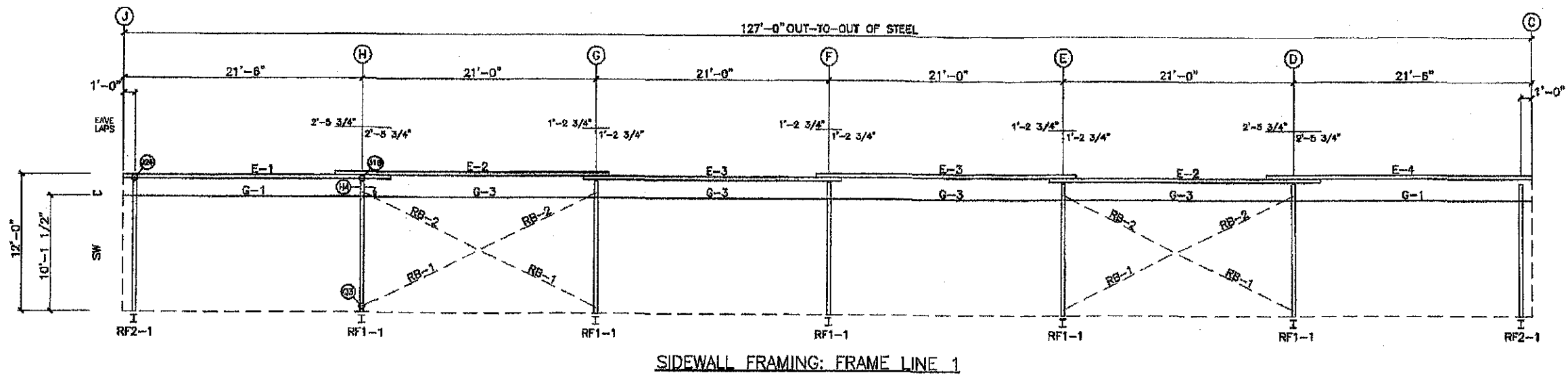
GENERAL NOTES:
 1. Pinnacle standard trim lap is 3 inches max.
 2. Pinnacle pre-cuts wall panels at factory located openings as required.
 3. Slot girts in field for cable passage at flush walls as required.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	11/14/16	

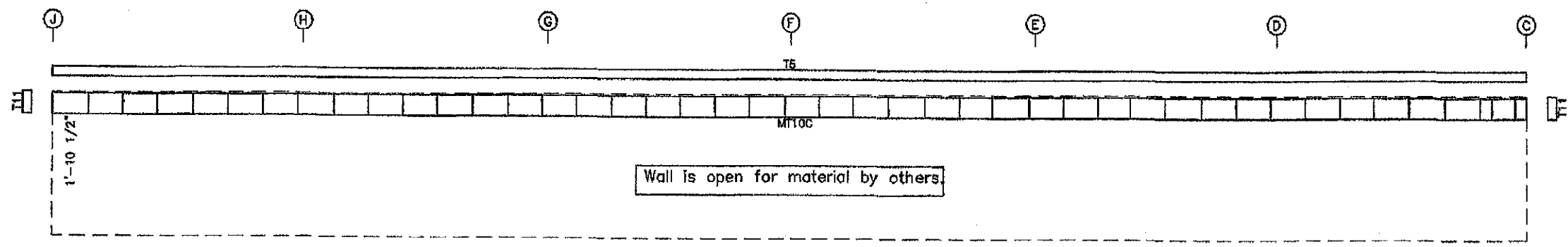


DESCRIPTION:	SIDEWALL FRAMING		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer:	SS	Checker:	DS
Designer:	SO		
Job No.:	161645A	Sheet:	E7
Issue:	P		

MEMBER TABLE	
FRAME LINE 1	
MARK	PART
E-1	8X25Z16
E-2	8X25Z16
E-3	8X25Z16
E-4	8X25Z16
G-1	8X25C16
G-2	W08541
G-3	8X25C16
G-4	W08541
RB-1	1/2" ROD
RB-2	1/2" ROD

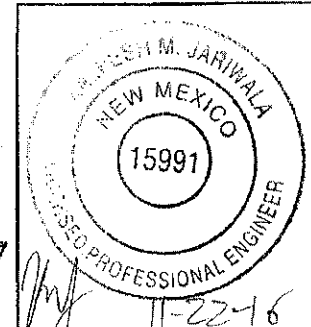


SIDEWALL FRAMING: FRAME LINE 1



SIDEWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 24 Co. PBR - Brite Red

FOR PERMITS ONLY



GENERAL NOTES:
 1. Pinnacle standard trim lap is 3 inches max.
 2. Pinnacle pre-cuts wall panels at factory located openings as required.
 3. Slot girts in field for cable passage at flush walls as required.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	11/14/16	



DESCRIPTION:	SIDEWALL FRAMING		
CUSTOMER:	7B BUILDING & DEVELOPMENT		
LOCATION:	ROSWELL, NM (CHAVES CO.)		
Detailer	SS	Checker	DS
Job No.	161645A	Sheet	EB
Designer	AS	Issue	P