PINNACLE STRUCTURES, INC.

IAS ACCREDITED Metal Building Systems AC 472

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Date: 12/18/2019

Customer:
VIA REAL ESTATE
13105 DOVER AVE.
LUBBOCK, TX 79424

Pinnacle Job #: 191766B
Project: CXCW - CORPUS CHRISTI
Project Location: CORPUS CHRISTI, TX 78415 (NUECES COUNTY)

Project Description:	Width	Length	L.EH	R.EH	L. Slope
	9'-0"	68'-0"	12'-0"	13'-1½"	1.5:12

This is to certify that the above referenced metal building and its components have been designed and fabricated by the metal building manufacturer, Pinnacle Structures Inc., in accordance with the information specified on the order documents. The specified design loads and criteria are applied in accordance with the **2015 International Building Code**. Pinnacle Structures Inc. is an IAS accredited manufacturer maintaining a quality system in compliance with both IAS AC472 criteria and the requirements of Chapter 17 of the International Building Code.

In addition to the dead load of the building components, the members are designed to the following basis:

Building Risk Category		II - Normal					
Collateral Loads*	C	3.00	psf	**Others:**			
Roof Live Load	L_r	20.00	psf	N/A			
(Reducible as permitted by code)							
Roof Snow Load Data							
Ground Snow Load	P_g	0.00	psf				
Flat-Roof Snow Load	P_f	0.00	psf				
Snow Exposure Factor	C_e	1.00					
Snow Importance Factor	I_s	1.00		Drift Surcharge Load(s)	P_d	N/A	
Thermal Factor	C_t	1.20		Width of Snow Drift(s)	w	N/A	

Wind Design Data						
Ultimate Design Wind Speed (3-second gust)	V_{ult}	155	mph	Wind Exposure	C	
Nominal Design Wind Speed	V_{asd}	120	mph	Internal Pressure Coefficient	±0.00	
Rain Intensity	i	10.0	in/hr			

Earthquake Design Data							
Analysis Procedure	*Equivalent Lateral Force Procedure*				Design Base Shear V		
Seismic Importance Factor	I_e	1.00			Transverse Direction	0.14	kips
Mapped Spectral Response Acceleration Parameters	S_s	0.064	S_1	0.021	Longitudinal Direction	0.14	kips
Design Spectral Response Acceleration Parameters	S_{DS}	0.069	S_{D1}	0.034			
Site Class D	Seismic design category			A			

Basic Seismic Force-Resisting Systems (SFRS)		C_s	R
Transverse	Steel Ordinary Moment Frame(s)	0.023	3.00
Left Endwall	Steel Moment Resisting Frame	0.023	3.00
Right Endwall	Steel Moment Resisting Frame	0.023	3.00
Front Sidewall	Torsionally Braced		
Back Sidewall	Steel Moment Resisting Frame(s)	0.023	3.00

Cs: *Seismic Response Coefficient.*

R: *Response Modification Coefficient.*

The buyer and/ or Engineer of Record for the Project is responsible to verify specified loads are in compliance with the local regulatory authorities and report any changes or deviations from the order documents to metal building manufacturer.

This project is designed as **open**. Exterior wall component and cladding materials not specifically supplied by Pinnacle Structures, Inc. should be designed to withstand 39.99/-39.99 psf in the field zone. Additional wind pressure / suction for other zones are available upon request.

*This project is designed for this collateral loading. Suspension of any load-inducing system in excess of this loading is prohibited without consultation with the manufacturer to determine structural reinforcement, if required, to safely support supplemental loads.

This project is designed using metal building manufacturer's standard serviceability standards in accordance with 2012 MBMA Manual criteria unless specified otherwise on the order documents.

This Letter of Certification applies solely to the structural framing and its component parts as furnished by the metal building manufacturer and as specified in the contract.

The undersigned engineer does not serve as or represent the Engineer of Record for the overall project.

Sincerely,

STATE OF TEXAS
ALFRED U. NKUNGA
101574
LICENSED PROFESSIONAL ENGINEER

JAN 22 2020

awilliams V 190905

GENERAL NOTES

1. This structure has been designed in accordance with the 2012 AISI NAUS Cold Formed Steel Design Manual and the AISC (14th Edition, ASD) Steel Construction Manual.

2. Fabrication shall be accordance with Pinnacle Standards in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1".

3.

Materials	ASTM Designation	Minimum Yield
Hot Rolled Angle	A36	Fy = 36 ksi
Structural Steel Plate	A572, A529, A1011	Fy = 55 ksi
Cold Formed Shapes	A1011/(A653 Galvanized)	Fy = 55 ksi
Cable Bracing	A475 (7-Wire Strand)	Ex. High Strength
Rod Bracing	A529 - GR 50	Fy = 50 ksi
Roof & Wall Sheeting	A792 26 GA	80 ksi, Class 1
	A792 24 and 22 GA	50 ksi, Class 2
High Strength Bolts	A325-Group A/(A490-Group B)	
Pipe	A53, Gr. B	Fy = 35 ksi
Round Structural Tubing	A500, Gr. B	Fy = 42 ksi
Shaped Structural Tubing	A500, Gr. B	Fy = 46 ksi
Hot Rolled Shapes	A572, A992, A529 Gr. 50	Fy = 50 ksi
Hot Rolled Shapes	A36	Fy = 36 ksi

4. Shop primer paint is a rust inhibitive primer which meets the end performance of SSPC-Paint 15: Steel Joist Shop Primer/ Metal Buiding Primer and is maroon oxide in color. This paint is not intended for long term exposure to the elements. Pinnacle Structures, Inc. is not responsible for any deterioration of the shop primer as a result of improper handling or storage. Pinnacle will not be responsible for any field applied paint and or coatings. (Section 7.17 AISC code of Standard Practice for Steel Buildings & Bridges, 13th Edition).

5. Bolts for the construction of Pinnacle Structures, Inc. material shall be as follows:
   - All secondary member connections - 1/2" x 1 1/4" A307 unless noted
   - Bearing frame endwall connections - A325
   - Main frame connections - A325 as shown on drawings

6. Connections Using High Strength Structural Bolts:

All high strength bolts are A325-N, unless noted otherwise. High strength structural bolts are supplied without washers, unless noted otherwise. Bolt length shall be such that the end of the bolt extends beyond or is at least flush with the outer face of the nut, when properly installed. All bolted connections, unless noted, are designed as bearing type connection with bolt threads not excluded from the shear plane.

A325-N High Strength Structural Bolts:
Snug-Tightened connections are permitted with A325-N bolts, except for these cases:
- Where crane beams and rigid frame connections in crane buildings are present
- In Slip-Critical Connections
- If noted in the erection drawings otherwise

For these exceptions, Turn-of-the-Nut method must be used.

A490 High Strength Structural Bolts:
A490 structural bolts shall be tightened using the Turn-of-the-Nut method. Snug-Tightened connections are not permitted with A490 bolts.

Tightening Methods:

Snug-Tightened Joint: A condition in which the tightness that exists when all of the plies in a connection have been pulled into firm contact by the bolts in the joint and all of the bolts in the joint have been tightened sufficiently to prevent the removal of the nuts without the use of a wrench, in accordance with the 14th Edition of AISC "Specification for Structural Joints Using High-Strength Bolts", per Section 8.1.

Turn-of-the-Nut method in is to be performed in accordance with the 14th Edition AISC "Specification for Structural Joints Using High-Strength Bolts" per Section 8.2.1.

7. All Bracing shown and provided by Pinnacle for this building is required for transferring building loads to the foundation and shall be installed by the erector as a permanent part of the structure. Cable/Rod bracing is designed for structural loads only and is not designed to plumb the building. The cable/rod bracing shall be taut, tighten to remove sag only. Bracing shall not be over-tighten. If additional bracing is required for stability during erection, it shall be the erectors responsibility to determine the amount of such bracing and to procure and install as necessary.

8. Soil profile type is determined by the foundation Engineer per local code.

9. Building Codes Require Consideration of Snow Surcharges for Any Lower Roof of a Structure Located within 20 Feet of a Higher Structure. Information Supplied to Pinnacle Structures Does Not Indicate the Presence of a Shadowing Structure within this 20 Foot Envelope. Therefore Snow Surcharges Have Not Been Considered in this Design Unless Noted Otherwise.

PINNACLE™ STRUCTURES, INC.

P.O. Box 1268
Cabot, AR 72023
Phone: (501) 941-3929 or (800) 201-1534
Fax: (501) 941-2675

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

IAS ACCREDITED® AC 472

JAN 22 2020
STATE OF TEXAS
ALFRED U. NKUNGA
101574
LICENSED
PROFESSIONAL ENGINEER

DRAWING PACKAGE FOR:

Customer: VIA REAL ESTATE

Job Number: 191766B

Project: CXCW - CORPUS CHRISTI

Project Location: CORPUS CHRISTI, TX (NUECES CO)

Project Description:

Width	Length	B.EH	F.EH	L. Slope
9'-0"	68'-0"	12'-0"	13'-1 1/2"	1.5:12

DESIGN REQUIREMENTS

Building Code: IBC 2015

Building Risk Category: II - Normal
Collateral Load:* 3
Roof Live Load: 20.00
Tributary Reduction: Yes

Roof Snow Load Data
Ground Snow Load (P_g): 0.00 psf
Flat Roof Snow Load (P_f): 0.00 psf
Snow Exposure Factor (C_e): 1.00
Snow Importance Factor (I_s): 1.00
Thermal Factor (C_t): 1.20

Wind Design Data
Ultimate Design Wind Speed : 155 mph (3 Second Gust) 1 REVISED WIND LOADING 1/22/20
Nominal Design Wind Speed : 120 mph 1
Internal Pressure Coefficient: ± 0.00
Wind Exposure: C

Earthquake Design Data:
Analysis Procedure - Equivalent LateraL Force Procedure
Seismic Importance Factor: Ie 1.00
Mapped Spectral Response Acceleration Parameters: Ss 0.064 S1 0.021
Design Spectral Response Acceleration Parameters: SDs 0.069 SD1 0.034
Site Class : D Seismic Design Category : A

Design Base Shear V
Transverse Direction : 0.14 kips 1
Longitudinal Direction : 0.14 kips 1

Revised

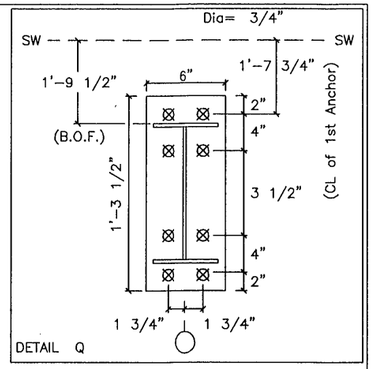
Basic Seismic Force- Resisting Systems (SFRS)		Cs	R
Transverse	Steel Ordinary Moment Frame(s)	0.023	3.00
Left Endwall	Steel Moment Resisting Frame(s)	0.023	3.00
Right Endwall	Steel Moment Resisting Frame(s)	0.023	3.00
Front Sidewall	Torsional Braced		
Back Sidewall	Steel Moment Resisting Frame(s)	0.023	3.00

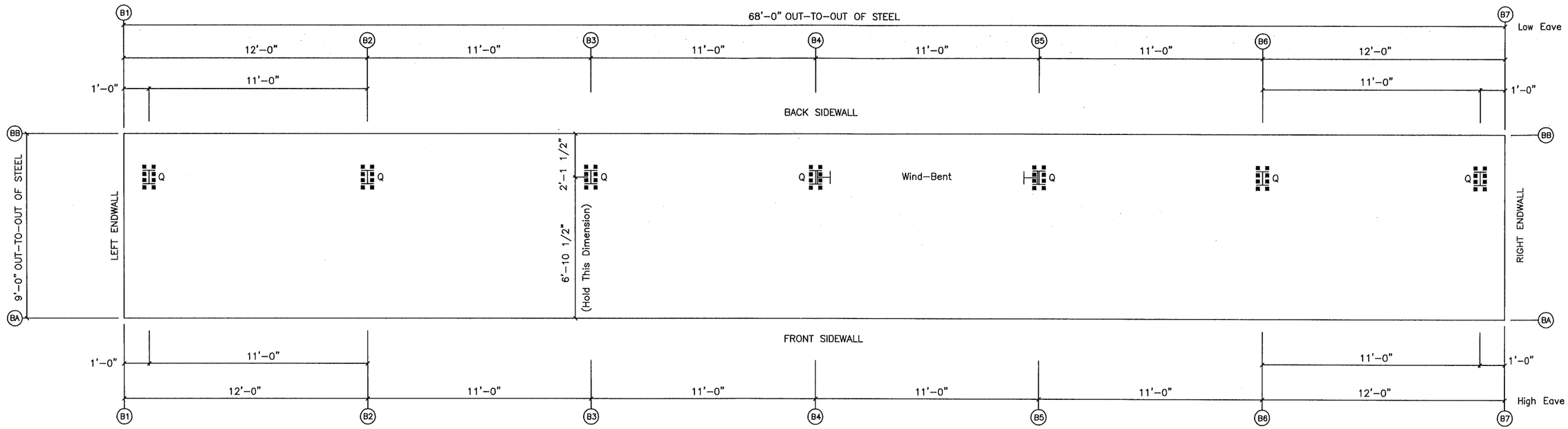
Other: N/A

Exterior wall component & cladding materials not specifically supplied by P.S.I. should be designed to withstand 35.00 /-35.00 psf in the field zone.

*This project is designed for this collateral loading. Suspension of any load-inducing system in excess of this loading is prohibited without consultation with the manufacturer to determine structural reinforcement, if required, to safely support supplemental loads.

NOTE: AB LAYOUT SHOWN 180° FROM ARCH. DWG'S.



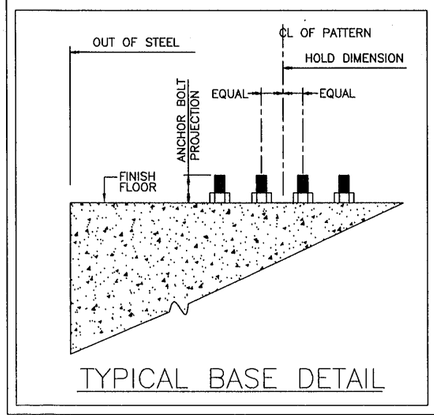


ANCHOR ROD PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

JAN 22 2020

Revised



TYPICAL BASE DETAIL

BLDG B

BLDG C

BLDG A

ANCHOR ROD SUMMARY				
Qty	Locate	Dia (in)	Type	Proj (in)
☒ 56	Frame	3/4"	F1554 – GR55	3.00

⚠1 REVISED WIND LOADING 1/22/20

GENERAL NOTES

- -These drawings are NOT to scale.
- -Pinnacle's steel line is shown.
- -A sheeting notch or brick ledge, if used, must be added to determine the out of concrete.
- -Wall panels shall be held 1/4" above the sheet notch and/or base trim.
- -Attachment of material by others to Pinnacle steel is the responsibility of others.

STATE OF TEXAS
ALFRED U. NKUNGA
101574
LICENSED
PROFESSIONAL ENGINEER

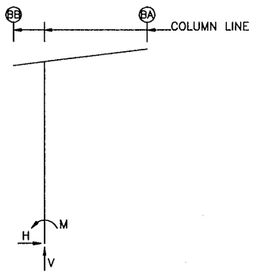
1SSUE	DESCRIPTION	DATE	MARK
0	CONSTRUCTION	1/03/20	
1	REVISED	1/22/20	

PINNACLE
STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ANCHOR ROD PLAN
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

Detailer	DS	Checker	MG	Designer	AW
Job No.	191766B	Sheet	F01	Issue	1

FRAME LINES: B1 B2 B3 B4 B5 B6 B7



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
1*	℄ 2.1	Moment connection, see table.						4	0.750	6.000	15.50	0.375	0.0

1* Frame lines: B1 B2 B3 B6 B7

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
4*	℄ 2.1	Moment connection, see table.						4	0.750	6.000	15.50	0.375	0.0

4* Frame lines: B4 B5

RIGID FRAME: BASIC COLUMN REACTIONS (k ,f-k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Dead Mom	Collateral Horiz	Collateral Vert	Collateral Mom	Live Horiz	Live Vert	Live Mom	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Left1 Mom
1*	℄ 2.1	0.0	0.6	0.89	0.0	0.3	0.74	0.0	2.0	4.88	0.4	-3.2	-15.56
4*	℄ 2.1	0.0	0.6	0.89	0.0	0.3	0.74	0.0	2.0	4.88	0.4	-3.2	-15.56

Frame Line	Column Line	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Right1 Mom	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Left2 Mom	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Right2 Mom	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long1 Mom
1*	℄ 2.1	0.4	-3.2	-9.51	0.4	-3.2	-10.30	0.4	-3.2	-14.60	0.6	-4.5	-17.49
4*	℄ 2.1	0.4	-3.2	-9.51	0.4	-3.2	-10.30	0.4	-3.2	-14.60	-0.4	-3.0	-2.45

Frame Line	Column Line	Wind_Long2 Horiz	Wind_Long2 Vert	Wind_Long2 Mom	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Left Mom	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Right Mom	Seismic_Long Horiz	Seismic_Long Vert	Seismic_Long Mom
1*	℄ 2.1	-0.2	1.9	7.37	0.0	0.0	0.23	0.0	0.0	-0.23	0.0	0.0	0.00
4*	℄ 2.1	0.6	1.6	-3.04	0.0	0.0	0.23	0.0	0.0	-0.23	-0.1	0.0	0.80

1* Frame lines: B1 B2 B3 B6 B7
4* Frame lines: B4 B5

GENERAL NOTES

1. ANCHOR RODS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
2. FOUNDATION DESIGN AND ANCHOR RODS LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
3. THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR ROD SUMMARY TABLE REPORTS THE ROD DIAMETERS.
4. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
5. ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8.

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	B1								(h)
F_SW	BA	Torsional Bracing Used							
R_EW	B7								(h)
B_SW	BB	B4,B5	1.0	1.9	0.1	0.1			(b)

(b)Wind bent in bay, base above finish floor
(h)Rigid frame at endwall

RIGID FRAME: REACTIONS FOR FRAME LINE : 1*

Load Id	Col. ℄ 25.50 Horiz	Vert	Moment
1	-0.1	3.2	8.60
2	0.2	-1.6	-8.81
3	0.3	-2.4	-9.96
4	-0.1	1.5	4.96
5	0.0	2.8	6.51
6	0.0	0.9	1.62
7	0.0	0.6	0.89
8	0.0	2.3	5.29
9	0.2	-1.4	-8.45
10	0.2	-1.4	-4.82
11	0.2	-1.4	-5.29
12	0.2	-1.4	-7.87
13	0.3	-2.1	-9.61
14	0.3	-2.1	-9.61
15	-0.1	1.7	5.31
16	-0.1	1.7	5.31
17	0.2	0.9	-1.72
18	0.2	0.9	1.01
19	0.2	0.9	0.65
20	0.2	0.9	-1.28
21	0.3	0.3	-2.58
22	0.3	0.3	-2.58
23	-0.1	3.2	8.60
24	0.2	-0.6	-5.38
25	0.2	-0.6	-2.65
26	0.2	-0.6	-3.01
27	0.2	-0.6	-4.95
28	0.3	-1.2	-6.25
29	0.3	-1.2	-6.25
30	-0.1	1.7	4.94
31	-0.1	1.7	4.94
32	0.2	-1.6	-5.17
33	0.2	-1.6	-5.65
34	0.2	-1.6	-8.23
35	0.3	-2.4	-9.96
36	-0.1	1.5	4.96
37	0.0	0.9	1.80
38	0.0	0.9	1.48
39	0.0	0.9	1.64
40	0.0	2.4	5.42
41	0.0	2.4	5.18
42	0.0	0.9	1.76
43	0.0	0.9	1.52
44	0.0	2.4	5.30
45	0.0	0.3	0.68
46	0.0	0.3	0.36
47	0.0	0.3	0.52

1* Frame lines: B2 B3 B6 B7

RIGID FRAME: REACTIONS FOR FRAME LINE : 4*

Load Id	Col. ℄ 25.50 Horiz	Vert	Moment
1	0.3	3.1	3.92
2	0.2	-1.6	-8.81
3	-0.3	-1.5	-0.94
4	0.4	1.3	-1.29
5	0.0	2.8	6.51
6	0.0	0.9	1.62
7	0.0	0.6	0.89
8	0.0	2.3	5.29
9	0.2	-1.4	-8.45
10	0.2	-1.4	-4.82
11	0.2	-1.4	-5.29
12	0.2	-1.4	-7.87
13	-0.3	-1.3	-0.58
14	-0.3	-1.3	-0.58
15	0.4	1.5	-0.94
16	0.4	1.5	-0.94
17	0.2	0.9	-1.72
18	0.2	0.9	1.01
19	0.2	0.9	0.65
20	0.2	0.9	-1.28
21	-0.2	1.0	4.19
22	-0.2	1.0	4.19
23	0.3	3.1	3.92
24	0.2	-0.6	-5.38
25	0.2	-0.6	-2.65
26	0.2	-0.6	-3.01
27	0.2	-0.6	-4.95
28	-0.2	-0.5	0.52
29	-0.2	-0.5	0.52
30	0.3	1.6	0.26
31	0.3	1.6	0.26
32	0.2	-1.6	-5.17
33	0.2	-1.6	-5.65
34	0.2	-1.6	-8.23
35	-0.3	-1.5	-0.94
36	0.4	1.3	-1.29
37	0.0	0.9	1.80
38	0.0	0.9	1.48
40	0.0	2.4	5.42
41	0.0	2.4	5.18
42	0.0	0.9	1.76
43	0.0	0.9	1.52
45	0.0	0.3	0.68
46	0.0	0.3	0.36
48	0.0	0.9	2.20
49	0.0	0.9	1.08
50	0.0	2.4	5.73
51	0.0	2.4	4.88
52	0.0	0.9	2.07
53	0.0	0.9	1.22
54	0.0	0.3	1.08
55	0.0	0.3	-0.04

4* Frame lines: B4 B5

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

Width (ft)	= 9.0
Length (ft)	= 68.0
Eave Height (ft)	= 12.0/ 13.1
Roof Slope (rise/12)	= 1.5
Dead Load (psf)	= 2.2
Collateral Load (psf)	= 3.0
Live Load (psf)	= 20.0
Wind Speed (mph)	= 155.0
Wind Code	= IBC 15
Exposure	= C
Closed/Open	= O
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= A
Seismic Coeff (Fa*Ss)	= 0.10

5. Loading conditions are:
   1 Dead+Collateral+0.75Live+0.45Wind_Long2R
   2 0.6Dead+0.6Wind_Left1
   3 0.6Dead+0.6Wind_Long1R
   4 0.6Dead+0.6Wind_Long2R
   5 Dead+Collateral+Live
   6 Dead+Collateral
   7 Dead
   8 Dead+Collateral+0.75Live
   9 Dead+0.6Wind_Left1
   10 Dead+0.6Wind_Right1
   11 Dead+0.6Wind_Left2
   12 Dead+0.6Wind_Right2
   13 Dead+0.6Wind_Long1L
   14 Dead+0.6Wind_Long1R
   15 Dead+0.6Wind_Long2L
   16 Dead+0.6Wind_Long2R
   17 Dead+Collateral+0.75Live+0.45Wind_Left1
   18 Dead+Collateral+0.75Live+0.45Wind_Right1
   19 Dead+Collateral+0.75Live+0.45Wind_Left2
   20 Dead+Collateral+0.75Live+0.45Wind_Right2
   21 Dead+Collateral+0.75Live+0.45Wind_Long1L
   22 Dead+Collateral+0.75Live+0.45Wind_Long1R
   23 Dead+Collateral+0.75Live+0.45Wind_Long2L
   24 Dead+Collateral+0.45Wind_Left1
   25 Dead+Collateral+0.45Wind_Right1
   26 Dead+Collateral+0.45Wind_Left2
   27 Dead+Collateral+0.45Wind_Right2
   28 Dead+Collateral+0.45Wind_Long1L
   29 Dead+Collateral+0.45Wind_Long1R
   30 Dead+Collateral+0.45Wind_Long2L
   31 Dead+Collateral+0.45Wind_Long2R
   32 0.6Dead+0.6Wind_Right1
   33 0.6Dead+0.6Wind_Left2
   34 0.6Dead+0.6Wind_Right2
   35 0.6Dead+0.6Wind_Long1L
   36 0.6Dead+0.6Wind_Long2L
   37 1.01Dead+1.01Collateral+0.7Seismic_Left
   38 1.01Dead+1.01Collateral+0.7Seismic_Right
   39 1.01Dead+1.01Collateral
   40 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_Left
   41 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_Right
   42 1.01Dead+1.01Collateral+0.53Seismic_Left
   43 1.01Dead+1.01Collateral+0.53Seismic_Right
   44 1.01Dead+1.01Collateral+0.75Live
   45 0.59Dead+0.7Seismic_Left
   46 0.59Dead+0.7Seismic_Right
   47 0.59Dead
   48 1.01Dead+1.01Collateral+0.7Seismic_LongL
   49 1.01Dead+1.01Collateral+0.7Seismic_LongR
   50 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_LongL
   51 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_LongR
   52 1.01Dead+1.01Collateral+0.53Seismic_LongL
   53 1.01Dead+1.01Collateral+0.53Seismic_LongR
   54 0.59Dead+0.7Seismic_LongL
   55 0.59Dead+0.7Seismic_LongR

JAN 22 2020

STATE OF TEXAS
ALFRED U. NKUNGA
107574
LICENSED
PROFESSIONAL ENGINEER

Revised

1 REVISED WIND LOADING 1/22/20

ISSUE	DESCRIPTION	DATE	MARK
0	CONSTRUCTION	1/03/20	
1	REVISED	1/22/20	

PINNACLE STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION: ANCHOR ROD REACTIONS & DETAILS
CUSTOMER: VIA REAL ESTATE
LOCATION: CORPUS CHRISTI, TX (NUECES CO)

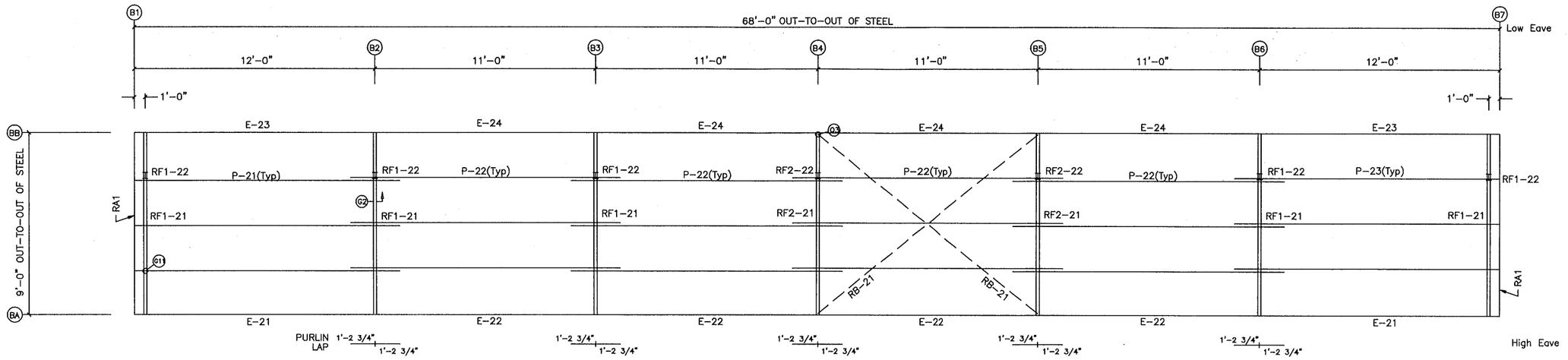
Detailer	Checker	Designer
DS	MG	AW

Job No. 191766B | Sheet F02 | Issue 1

NOTE:
Alternate Arrows ▽-△
Up And Down From Bay
To Bay For Purlins To Lap.

MEMBER TABLE ROOF PLAN	
MARK	PART
P-21	8X25Z16
P-22	8X25Z16
P-23	8X25Z16
E-21	8ES14-2
E-22	8ES14-2
E-23	8ES14-2
E-24	8ES14-2
RB-21	1/2"ROD

NOTE: AB LAYOUT SHOWN 180° FROM ARCH. DWG'S.

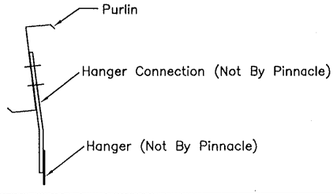


ROOF FRAMING PLAN

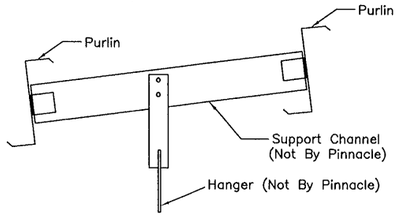
COLLATERAL LOAD NOTE:

Roof purlin has been designed for the collateral load listed on the cover. The total applied loads due to ceiling panels, ducts, sprinkler distribution lines, electrical equipment, conduit, fireproofing, other piping or mechanical loads cannot exceed this maximum uniform load. Pinnacle Structures, Inc. is not responsible for lateral or longitudinal bracing of suspended members subject to lateral seismic or wind loading.

Loads supported directly from the purlins must have connections through the web of the purlin.



Loads supported between purlins must be supported such that the loads are applied to the webs of the purlins.



BLDG B

BLDG C

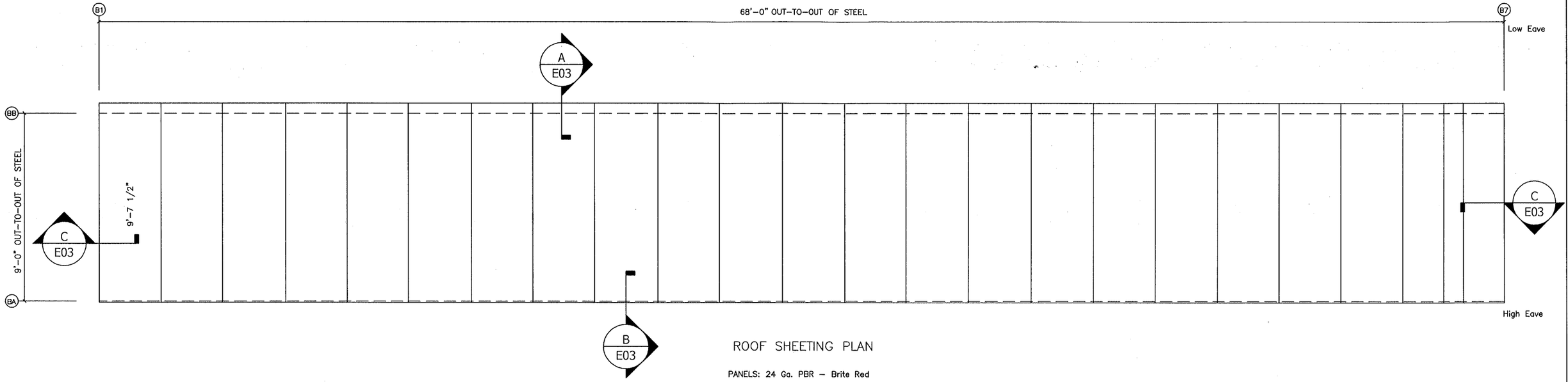
BLDG A

1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ROOF FRAMING PLAN
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

Detailer	DS	Checker	MG	Designer	AW
Job No.	191766B	Sheet	E01	Issue	P1



ROOF SHEETING PLAN

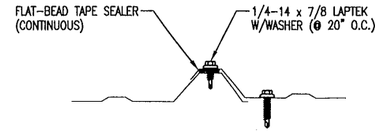
PANELS: 24 Ga. PBR – Brite Red

1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

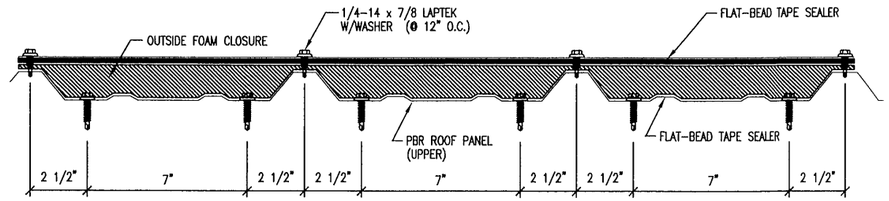
PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ROOF SHEETING
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

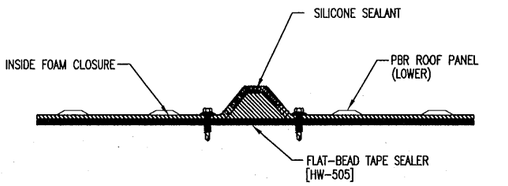
Detailer	Checker	Designer
DS	MG	AW
Job No. 191766B	Sheet E02	Issue P1



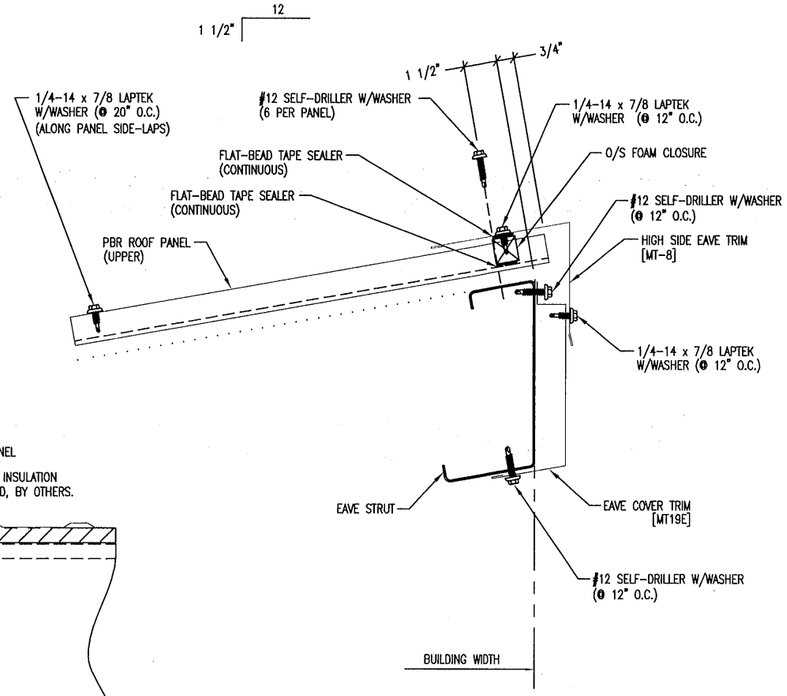
SECTION THROUGH PANEL SIDELAP



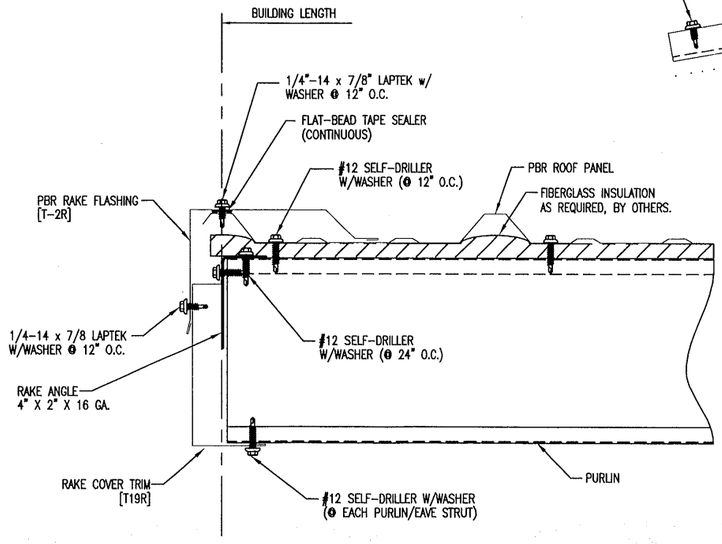
VIEW THROUGH HIGH-SIDE EAVE



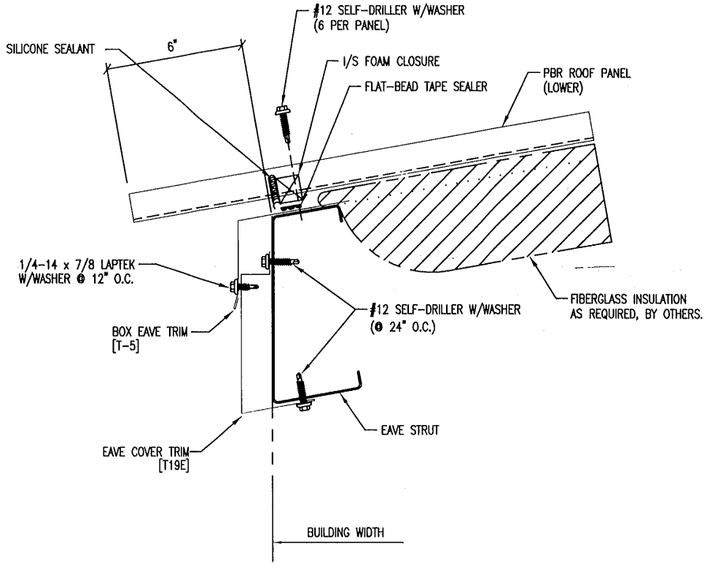
SECTION THROUGH EAVE END



B E03 SECTION DETAIL



C E03 SECTION DETAIL



A E03 SECTION DETAIL

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

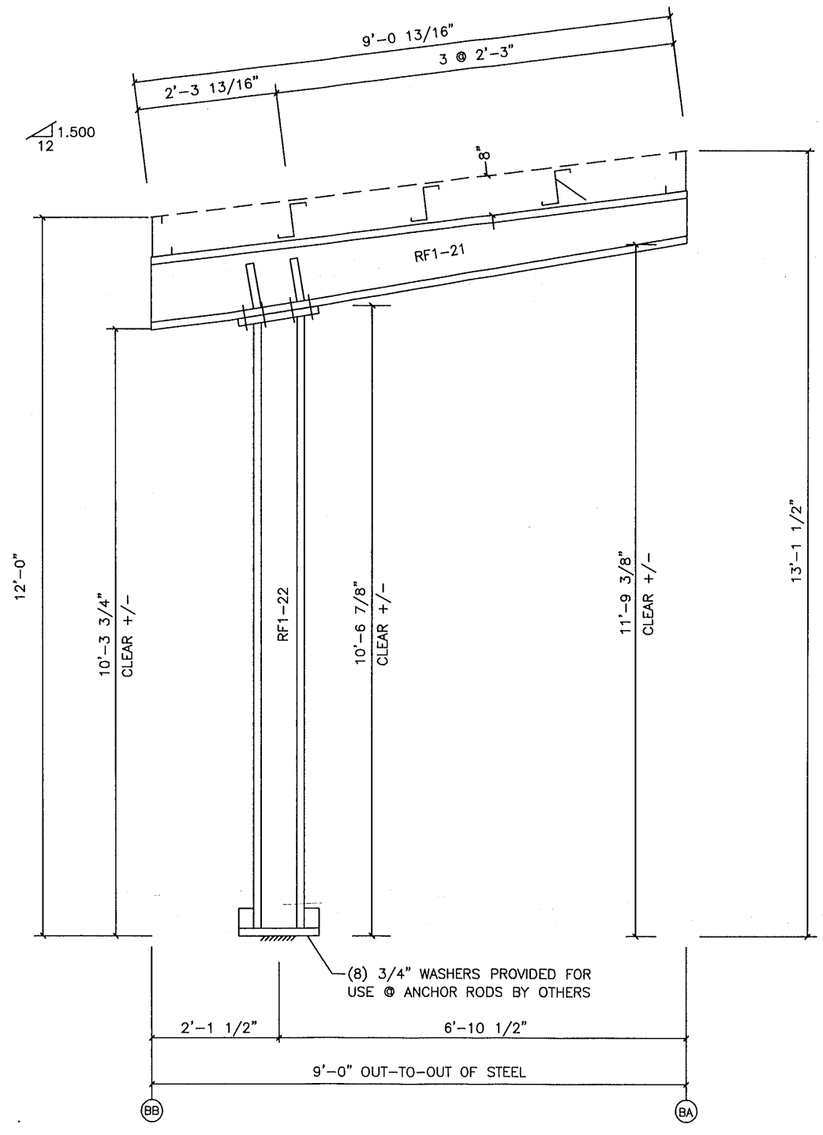
PINNACLE STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	PBR ROOF PANEL DETAILS
CUSTOMER:	VIA REAL ESTATE, LLC
LOCATION:	CORPUS CHRISTI, TX (NUECES CO.)

Detailer	DS	Checker	MG	Designer	AW
Job No.	191766B	Sheet	E03	Issue	P1

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF1–22	8	A325	5/8"	1 1/2"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1–21	11.5/11.5	0.135	49.9	6 x 1/4" x 108.8	6 x 3/8" x 49.9
RF1–22	11.5/ 7.5	0.135	60.4		6 x 1/4" x 59.5
	W08662				



RIGID FRAME ELEVATION: FRAME LINE B1 B2 B3 B6 B7

1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

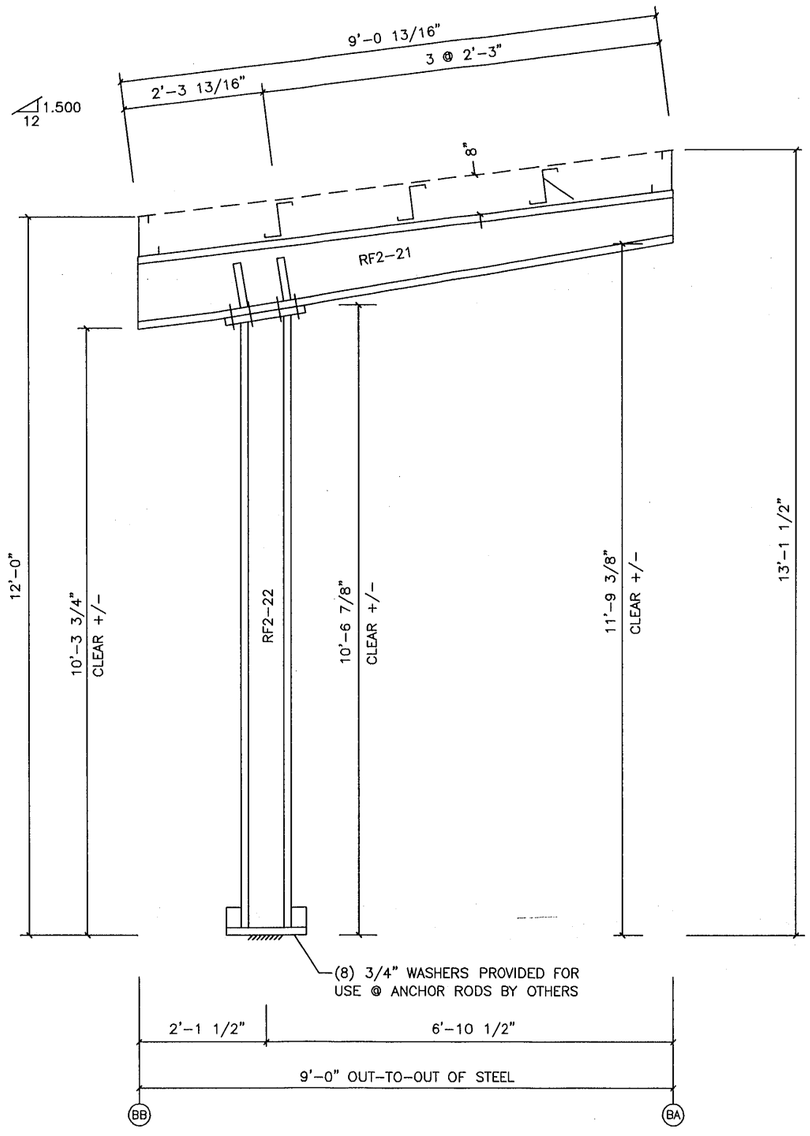
PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941–3929

DESCRIPTION:	RIGID FRAME ELEVATION
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

Detailer	Checker	Designer
DS	MG	AW
Job No. 191766B	Sheet E04	Issue P1

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF2–22	8	A325	5/8"	1 1/2"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF2–21	11.5/11.5	0.135	49.9	6 x 1/4" x 108.8	6 x 3/8" x 49.9
	11.5/ 7.5	0.135	60.4		6 x 1/4" x 59.5
RF2–22	W08662				



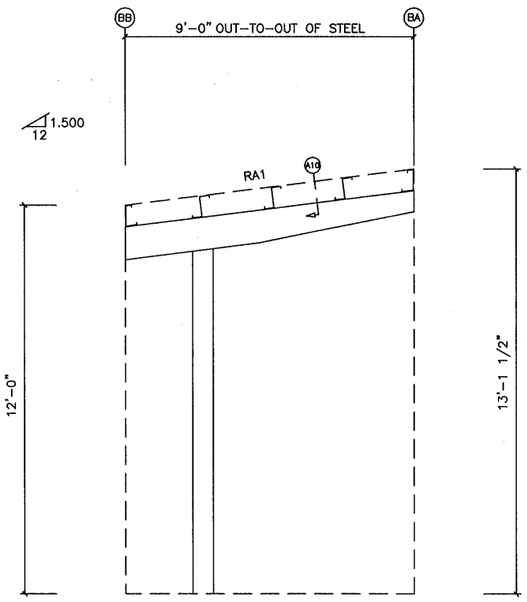
RIGID FRAME ELEVATION: FRAME LINE B4 B5

1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

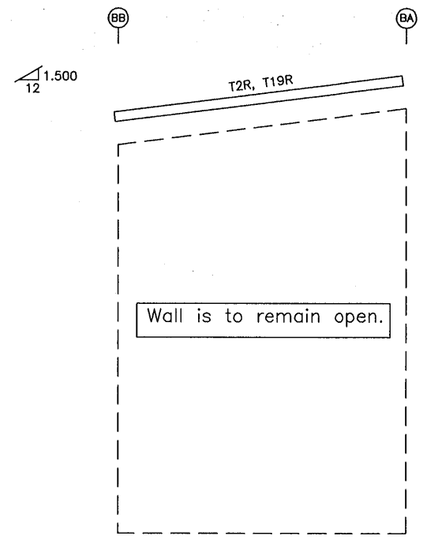
PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941–3929

DESCRIPTION:	RIGID FRAME ELEVATION
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

Detailer	Checker	Designer
DS	MG	AW
Job No. 191766B	Sheet E05	Issue P1



ENDWALL FRAMING: FRAME LINE B1



ENDWALL SHEETING & TRIM: FRAME LINE B1

GENERAL NOTES:
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

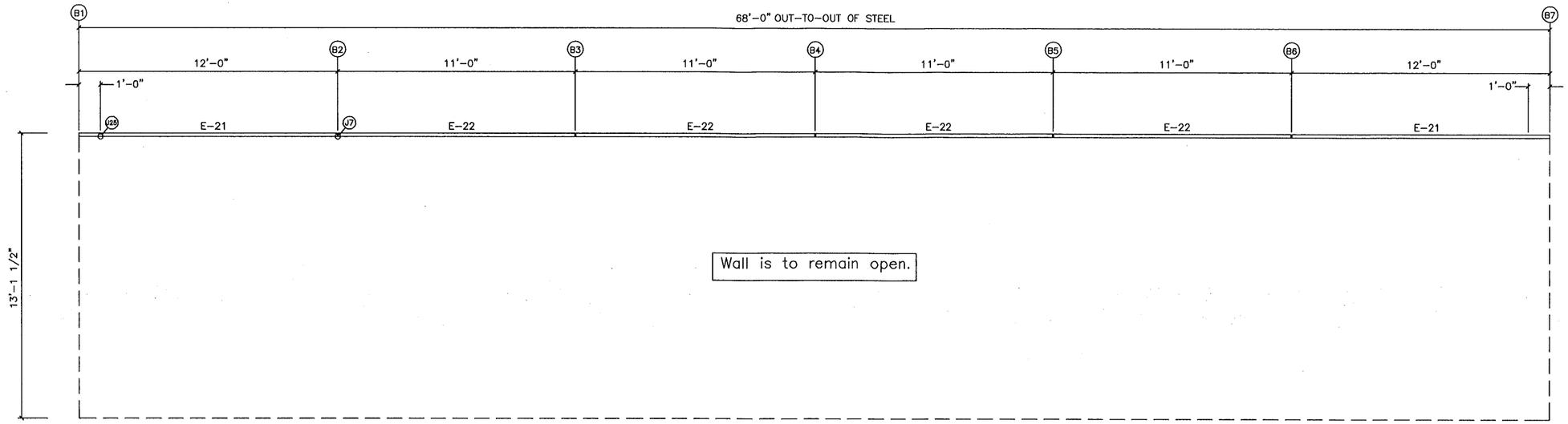
ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

PINNACLE STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

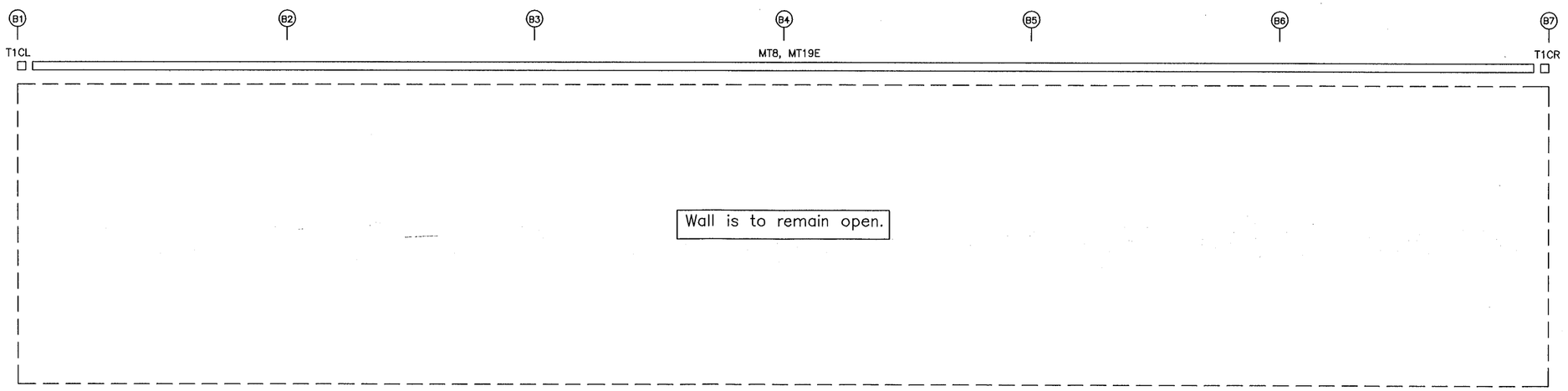
DESCRIPTION:	ENDWALL FRAMING
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

Detailer	Checker	Designer
DS	MG	AW
Job No. 191766B	Sheet E06	Issue P1

MEMBER TABLE FRAME LINE A	
MARK	PART
E-21	8ES14-2
E-22	8ES14-2



SIDEWALL FRAMING: FRAME LINE BA



SIDEWALL SHEETING & TRIM: FRAME LINE BA

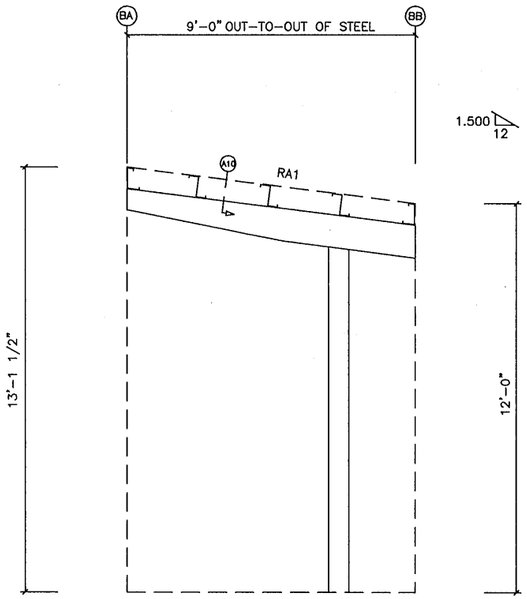
GENERAL NOTES:
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

1SSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

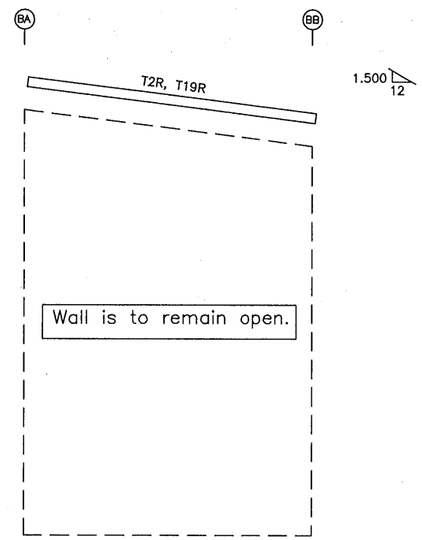
PINNACLE TM STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	SIDEWALL FRAMING
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

Detailer	DS	Checker	MG	Designer	AW
Job No.	191766B	Sheet	E07	Issue	P1



ENDWALL FRAMING: FRAME LINE B7



ENDWALL SHEETING & TRIM: FRAME LINE B7

GENERAL NOTES:
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

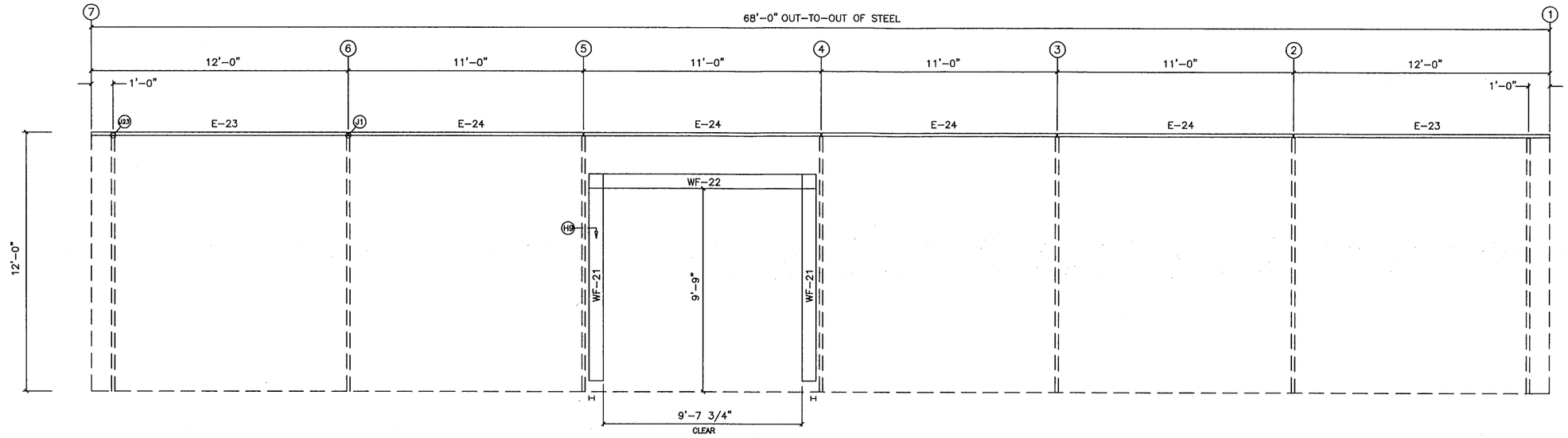
PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ENDWALL FRAMING
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

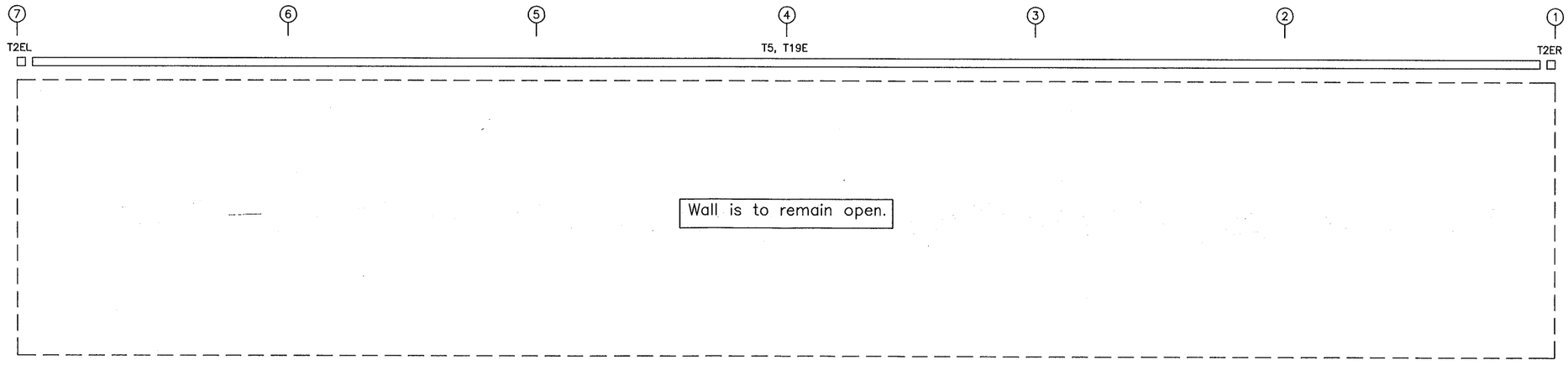
Detailer	Checker	Designer
DS	MG	AW
Job No. 191766B	Sheet E08	Issue P1

BOLT TABLE FRAME LINE B				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-21 – WF-22	8	A325	3/4"	1 3/4"
WF-21 – ------	6	A325	5/8"	1 1/2"

MEMBER TABLE FRAME LINE B	
MARK	PART
WF-21	W08641
WF-22	W08641
E-23	8ES14-2
E-24	8ES14-2



SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

GENERAL NOTES:
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	1/03/20	
P1	PERMIT	1/22/20	

PINNACLE STRUCTURES, INC.
PO BOX 1268 – CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	SIDEWALL FRAMING
CUSTOMER:	VIA REAL ESTATE
LOCATION:	CORPUS CHRISTI, TX (NUECES CO)

Detailer	Checker	Designer
DS	MG	AW
Job No. 191766B	Sheet E09	Issue P1