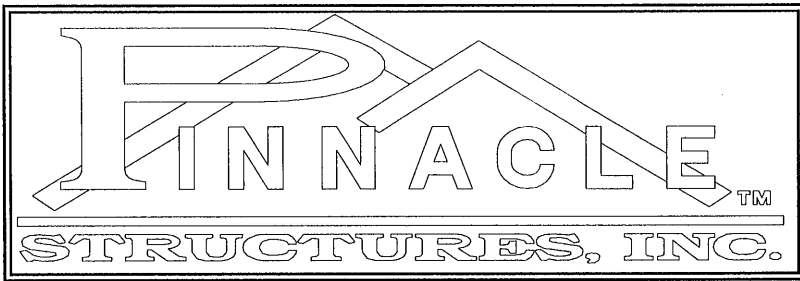


12



TRANSMITTAL

PO BOX 1268
CABOT, AR. 72023
PHONE: 501-941-3929
FAX: 501-941-2675

TO: VIA REAL ESTATE, LLC
13105 DOVER AVE.
LUBBOCK, TX. 79424

DATE: 6/1/2020
JOB #: 201287C

CONTACT: TYLER HOUK

WE ARE SENDING YOU THE FOLLOWING ITEMS:

1 SETS OF ANCHOR BOLT PLANS SEALED
 WITH CERT. SHEET

SET OF REVISED ANCHOR BOLT PLANS SEALED
 WITH CERT. SHEET

1 SETS OF PERMIT DRAWINGS NOT FOR CONSTRUCTION SEALED
 WITH CERT. SHEET

SETS OF APP'L ANCHOR DWGS NOT FOR CONSTRUCTION SEALED
 WITH CERT. SHEET

Roof Panel Type: SSR PBR
(If SSR is checked, an installation manual will be provided.)

SETS OF FINAL DRAWINGS FOR CONSTRUCTION SEALED
 WITH CERT. SHEET

SETS OF ENGINEERING CALCULATIONS SEALED

SET OF: _____

EMAIL DWGS TO: tyler@7bdev.com
derrick@7bdev.com

CC: JOSH, JOHN G. & DON

Notes: _____

Permit Drawings for DSXCW Bldg # C in Laredo, TX. (Webb Co.)

PINNACLE STRUCTURES, INC

BY: DONALD SALE

US MAIL 2ND DAY OVERNIGHT CUSTOMER PICK-UP

Date: 5/26/2020

Customer:
VIA REAL ESTATE
13105 DOVER AVE.
LUBBOCK, TX 79424

Pinnacle Job #: 201287C
Project: DALE SHINE XPRESS CAR WASH
Project Location: LAREDO, TX 78041 (WEBB COUNTY)
Project Description: Width Length L.EH R.EH L. Slope
9'-0" 122'-0" 12'-0" 13'-1½" 1.5:12

This is to certify that the above referenced metal building and its components have been designed and fabricated by the metal building manufacturer, Pinnacle Structures Inc., in accordance with the information specified on the order documents. The specified design loads and criteria are applied in accordance with the **2012 International Building Code**. Pinnacle Structures Inc. is an IAS accredited manufacturer maintaining a quality system in compliance with both IAS AC472 criteria and the requirements of Chapter 17 of the International Building Code.

In addition to the dead load of the building components, the members are designed to the following basis:

Building Risk Category II - Normal
Collateral Loads* C 5.00 psf
Roof Live Load L_r 20.00 psf
(Reducible as permitted by code)

Others:
N/A

Roof Snow Load Data

Ground Snow Load P_g 0.00 psf
Flat-Roof Snow Load P_f 0.00 psf
Snow Exposure Factor C_e 1.00
Snow Importance Factor I_s 1.00
Thermal Factor C_t 1.00

Drift Surcharge Load(s) P_d N/A
Width of Snow Drift(s) w N/A

Wind Design Data

Ultimate Design Wind Speed (3-second gust) V_{ult} 115 mph Wind Exposure C
Nominal Design Wind Speed V_{asd} 89 mph Internal Pressure Coefficient ±0.00
Rain Intensity i 9 in/hr

Earthquake Design Data

Analysis Procedure **Equivalent Lateral Force Procedure**
Seismic Importance Factor I_e 1.00 Design Base Shear V
Mapped Spectral Response Acceleration Parameters S_s 0.052 S_1 0.018 Transverse Direction 1.98 kips
Design Spectral Response Acceleration Parameters S_{DS} 0.056 S_{D1} 0.028 Longitudinal Direction 0.64 kips
Site Class D Seismic design category A

Basic Seismic Force-Resisting Systems (SFRS)

		C_s	R
Transverse	Steel Ordinary Moment Frame(s)	0.044	1.25
Left Endwall	Steel Ordinary Moment Frame	0.044	1.25
Right Endwall	Steel Ordinary Moment Frame	0.044	1.25
Front Sidewall	Torsionally Braced		
Back Sidewall	Steel Moment Resisting Frame(s)	0.019	3.00

C_s : Seismic Response Coefficient.

R: Response Modification Coefficient.

The buyer and/ or Engineer of Record for the Project is responsible to verify specified loads are in compliance with the local regulatory authorities and report any changes or deviations from the order documents to metal building manufacturer.

This project is designed as **open**. Exterior wall component and cladding materials not specifically supplied by Pinnacle Structures, Inc. should be designed to withstand 22.01/-24.22 psf in the field zone. Additional wind pressure / suction for other zones are available upon request.

*This project is designed for this collateral loading. Suspension of any load-inducing system in excess of this loading is prohibited without consultation with the manufacturer to determine structural reinforcement, if required, to safely support supplemental loads.

This project is designed using metal building manufacturer's standard serviceability standards in accordance with 2012 MBMA Manual criteria unless specified otherwise on the order documents.

This Letter of Certification applies solely to the structural framing and its component parts as furnished by the metal building manufacturer and as specified in the contract.

The undersigned engineer does not serve as or represent the Engineer of Record for the overall project.

Sincerely,
ALFRED U. NKUNGA
101574
LICENSED PROFESSIONAL ENGINEER
STATE OF TEXAS
JUN 02 2020

GENERAL NOTES

- This structure has been designed in accordance with the 2007 AISI NAUS Cold Formed Steel Design Manual and the AISI (14th Edition, ASD) Steel Construction Manual.
- Fabrication shall be accordance with Pinnacle Standards in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1".
- Materials**

ASTM Designation	Minimum Yield
Hot Rolled Angle A36	Fy = 36 ksi
Structural Steel Plate A572, A529, A1011	Fy = 55 ksi
Cold Formed Shapes A1011/(A653 Galvanized)	Fy = 55 ksi
Cable Bracing A475 (7-Wire Strand)	Ex. High Strength
Rod Bracing A529 - GR 50	Fy = 50 ksi
Roof & Wall Sheeting A792 26 GA	80 ksi, Class 1
A792 24 and 22 GA	50 ksi, Class 2
High Strength Bolts A325-Group A/(A490-Group B)	
Pipe A53, Gr. B	Fy = 35 ksi
Round Structural Tubing A500, Gr. B	Fy = 42 ksi
Shaped Structural Tubing A500, Gr. B	Fy = 46 ksi
Hot Rolled Shapes A572, A992, A529 Gr. 50	Fy = 50 ksi
Hot Rolled Shapes A36	Fy = 36 ksi
- Shop primer paint is a rust inhibitive primer which meets the end performance of SSPC-Paint 15: Steel Joist Shop Primer/ Metal Buiding Primer and is maroon oxide in color. This paint is not intended for long term exposure to the elements. Pinnacle Structures, Inc. is not responsible for any deterioration of the shop primer as a result of improper handling or storage. Pinnacle will not be responsible for any field applied paint and or coatings. (Section 7.17 AISC code of Standard Practice for Steel Buildings & Bridges, 13th Edition).
- Bolts for the construction of Pinnacle Structures, Inc. material shall be as follows:

 - All secondary member connections - 1/2" x 1 1/4" A307 unless noted
 - Bearing frame endwall connections - A325
 - Main frame connections - A325 as shown on drawings
- Connections Using High Strength Structural Bolts:

All high strength bolts are A325-N, unless noted otherwise. High strength structural bolts are supplied without washers, unless noted otherwise. Bolt length shall be such that the end of the bolt extends beyond or is at least flush with the outer face of the nut, when properly installed. All bolted connections, unless noted, are designed as bearing type connection with bolt threads not excluded from the shear plane.

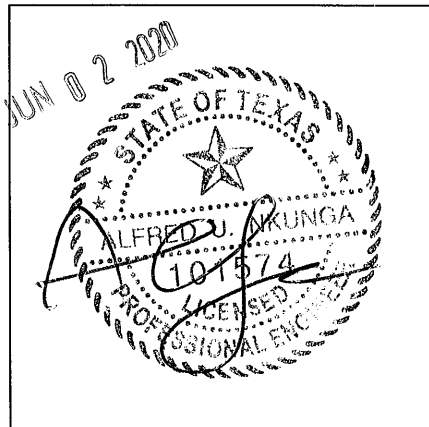
A325-N High Strength Structural Bolts:
Snug-Tightened connections are permitted with A325-N bolts, except for these cases:
 - Where crane beams and rigid frame connections in crane buildings are present
 - In Slip-Critical Connections
 - If noted in the erection drawings otherwise
 For these exceptions, Turn-of-the-Nut method must be used.

A490 High Strength Structural Bolts:
 A490 structural bolts shall be tightened using the Turn-of-the-Nut method. Snug-Tightened connections are not permitted with A490 bolts.

Tightening Methods:
Snug-Tightened Joint: A condition in which the tightness that exists when all of the plies in a connection have been pulled into firm contact by the bolts in the joint and all of the bolts in the joint have been tightened sufficiently to prevent the removal of the nuts without the use of a wrench, in accordance with the 14th Edition of AISC "Specification for Structural Joints Using High-Strength Bolts", per Section 8.1.
Turn-of-the-Nut method in is to be performed in accordance with the 14th Edition AISC "Specification for Structural Joints Using High-Strength Bolts" per Section 8.2.1.
- All Bracing shown and provided by Pinnacle for this building is required for transferring building loads to the foundation and shall be installed by the erector as a permanent part of the structure. Cable/Rod bracing is designed for structural loads only and is not designed to plumb the building. The cable/rod bracing shall be taut, tighten to remove sag only. Bracing shall not be over-tighten. If additional bracing is required for stability during erection, it shall be the erectors responsibility to determine the amount of such bracing and to procure and install as necessary.
- Soil profile type is determined by the foundation Engineer per local code.
- Building Codes Require Consideration of Snow Surcharges for Any Lower Roof of a Structure Located within 20 Feet of a Higher Structure. Information Supplied to Pinnacle Structures Does Not Indicate the Presence of a Shadowing Structure within this 20 Foot Envelope. Therefore Snow Surcharges Have Not Been Considered in this Design Unless Noted Otherwise.



P.O. Box 1268
 Cabot, AR 72023
 Phone: (501) 941-3929 or (800) 201-1534
 Fax: (501) 941-2675



DRAWING PACKAGE FOR:

Customer: VIA REAL ESTATE, LLC.
 Job Number: 201287C
 Project: DALE SHINE XPRESS CAR WASH
 Project Location: LAREDO, TX (WEBB CO)

Project Description:

Width	Length	B.EH	F.EH	L. Slope
9'-0"	122'-0"	12'-0"	13'-1 1/2"	1.5:12

DESIGN REQUIREMENTS

Building Code: IBC 2012

Building Risk Category: II - Normal
 Collateral Load: * 5.00
 Roof Live Load: 20.00
 Tributary Reduction: Yes

Roof Snow Load Data

Ground Snow Load (Pg): 0.00 psf
 Flat Roof Snow Load (Pf): 0.00 psf
 Snow Exposure Factor (Ce): 1.00
 Snow Importance Factor (Is): 1.00
 Thermal Factor (Ct): 1.00

Wind Design Data

Ultimate Design Wind Speed : 115 mph
 (3 Second Gust)
 Nominal Design Wind Speed : 89 mph
 Internal Pressure Coefficient: ± 0.00
 Wind Exposure: C

Earthquake Design Data:

Analysis Procedure - Equivalent Lateral Force Procedure
 Seismic Importance Factor: Ie 1.00
 Mapped Spectral Response Acceleration Parameters: Ss 0.052 S1 0.018
 Design Spectral Response Acceleration Parameters: Sds 0.056 Sd1 0.028
 Site Class : D Seismic Design Category : A

Design Base Shear V
 Transverse Direction : 1.98 kips
 Longitudinal Direction : 0.64 kips

Basic Seismic Force- Resisting Systems (SFRS)

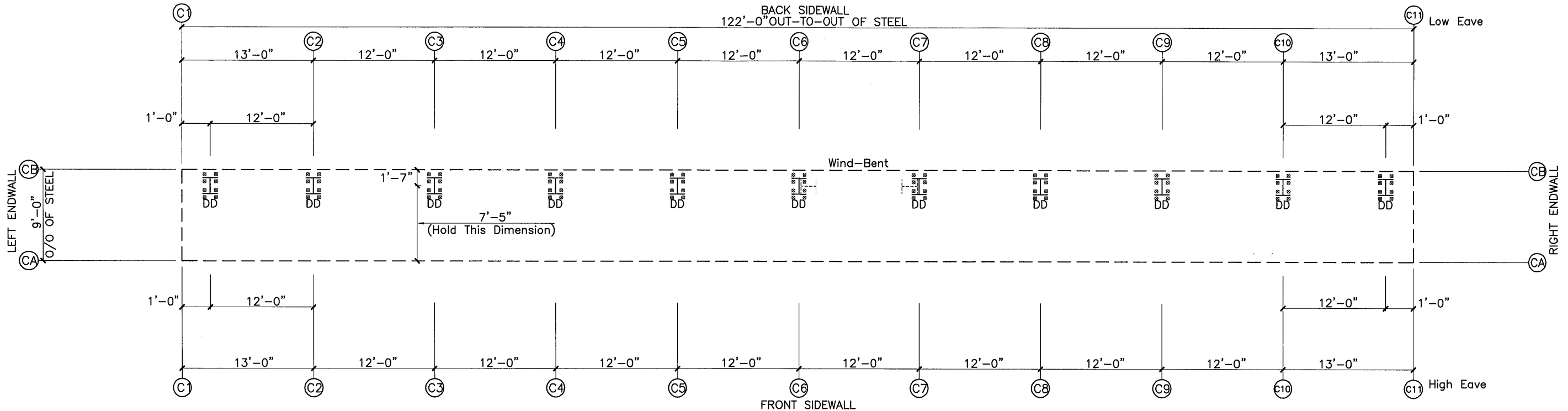
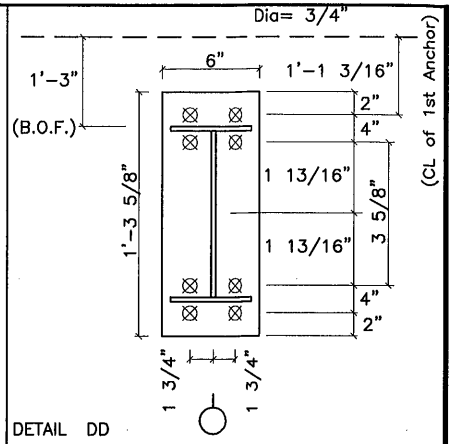
		Cs	R
Transverse	Steel Ordinary Moment Frame(s)	0.044	1.25
Left Endwall	Steel Ordinary Moment Frame	0.044	1.25
Right Endwall	Steel Ordinary Moment Frame	0.044	1.25
Front Sidewall	Torsional Bracing		
Back Sidewall	Steel Moment Resisting Frame(s)	0.019	3.00

Other: N/A

Exterior wall component & cladding materials not specifically supplied by P.S.I. should be designed to withstand 22.01 /-24.22 psf in the field zone.

*This project is designed for this collateral loading. Suspension of any load-inducing system in excess of this loading is prohibited without consultation with the manufacturer to determine structural reinforcement, if required, to safely support supplemental loads.

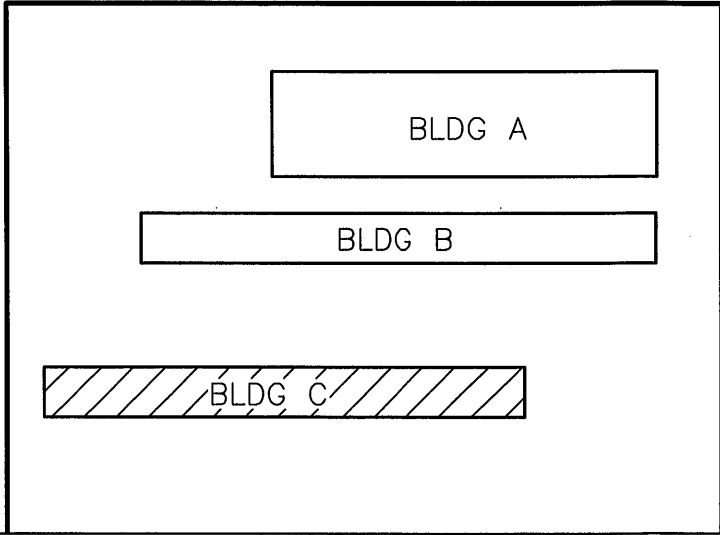
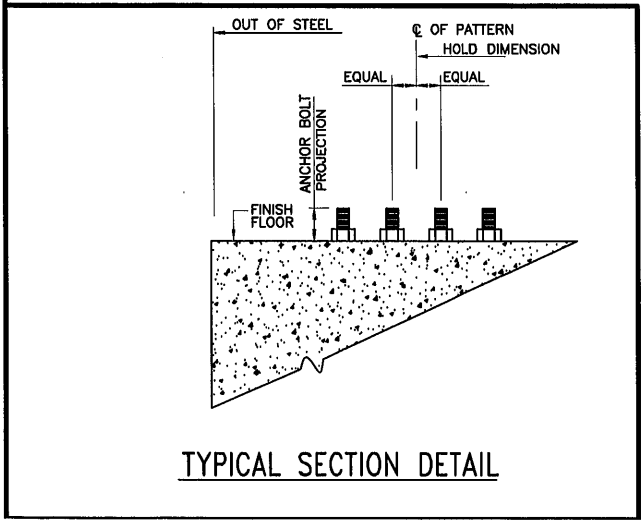
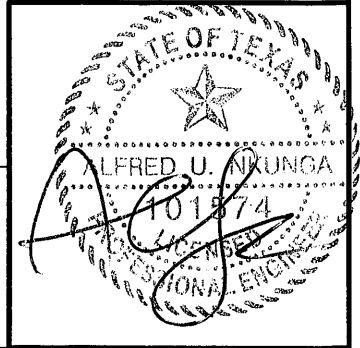
NOTE: AB LAYOUT SHOWN 180° FROM ARCH. DWG'S. NOTE FRAME LINE & COLUMN LINE MARKINGS



ANCHOR ROD PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

ANCHOR ROD SUMMARY				
Qty	Locate	Dia (in)	Type	Proj (in)
88	Frame	3/4"	F1554 - GR55	3.00

JUN 02 2020



GENERAL NOTES

- These drawings are NOT to scale.
- Pinnacle's steel line is shown.
- A sheeting notch or brick ledge, if used, must be added to determine the out of concrete.
- Wall panels shall be held 1/4" above the sheet notch and/or base trim.
- Attachment of material by others to Pinnacle steel is the responsibility of others.

ISSUE	DESCRIPTION	DATE	MARK
0	CONSTRUCTION	5/29/20	



DESCRIPTION: ANCHOR ROD PLAN			
CUSTOMER: VIA REAL ESTATE, LLC.			
LOCATION: LAREDO, TX (WEBB CO)			
Detailer SS	Checker DS	Designer KJK	
Job No. 201287C	Sheet F1	Issue 0	

RIGID FRAME: BASIC COLUMN REACTIONS (k, f-k)

Frame Line	Column Line	Dead	Live	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind_Long1	Wind_Long2	Seismic_Left	Seismic_Right	Seismic_Long	
C1*	1.6	0.0	0.6	1.17	0.0	0.6	1.71	0.0	2.2	6.77	0.3	-2.0	-10.82
C1*	1.6	0.3	-2.0	-7.04	0.2	-2.0	-7.52	0.2	-2.0	-10.21	-0.1	-2.8	-6.86
C1*	1.6	0.3	1.2	-0.34	-0.2	0.0	2.06	0.2	0.0	-2.06	-0.5	0.0	5.25

C1* Frame Lines: C1 C2 C3 C4 C5 C6 C7 C8 C9 C10 C11

BUILDING BRACING REACTIONS

Wall Loc	Col Line	± Reactions(k)	Panel Shear (lb/ft)	Note					
Line	Line	Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	Wind	Seis		
L_EW	C1							(h)	
F_SW	CA	Torsional Bracing Used							
R_EW	C11							(h)	
B_SW	CB	0.6	1.1	0.3	0.6			(b)	

(b) Wind bent in bay, base above finish floor
(h) Rigid frame at endwall

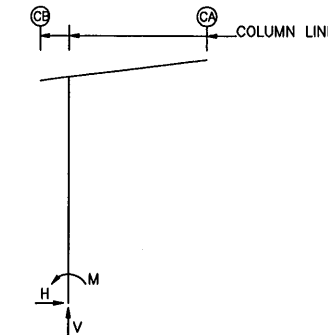
NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 9.0
 - Length (ft) = 122.0
 - Eave Height (ft) = 12.0/ 13.1
 - Roof Slope (rise/12) = 1.5
 - Dead Load (psf) = 2.2
 - Collateral Load (psf) = 5.0
 - Live Load (psf) = 20.0
 - Wind Speed (mph) = 115.0
 - Wind Code = IBC 12
 - Exposure = C
 - Closed/Open = 0
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = A
 - Seismic Coeff (Fa*Sa) = 0.08

5. Loading conditions are:

- Dead+Collateral+Live
- 0.6Dead+0.6Wind_Long1R
- 0.59Dead+0.7Seismic_LongL
- 0.59Dead+0.7Seismic_LongR
- Dead+Collateral
- Dead
- Dead+Collateral+0.75Live
- Dead+0.6Wind_Left1
- Dead+0.6Wind_Right1
- Dead+0.6Wind_Left2
- Dead+0.6Wind_Right2
- Dead+0.6Wind_Long1L
- Dead+0.6Wind_Long1R
- Dead+0.6Wind_Long2L
- Dead+0.6Wind_Long2R
- Dead+Collateral+0.75Live+0.45Wind_Left1
- Dead+Collateral+0.75Live+0.45Wind_Right1
- Dead+Collateral+0.75Live+0.45Wind_Left2
- Dead+Collateral+0.75Live+0.45Wind_Right2
- Dead+Collateral+0.75Live+0.45Wind_Long1L
- Dead+Collateral+0.75Live+0.45Wind_Long1R
- Dead+Collateral+0.75Live+0.45Wind_Long2L
- Dead+Collateral+0.75Live+0.45Wind_Long2R
- Dead+Collateral+0.45Wind_Left1
- Dead+Collateral+0.45Wind_Right1
- Dead+Collateral+0.45Wind_Left2
- Dead+Collateral+0.45Wind_Right2
- Dead+Collateral+0.45Wind_Long1L
- Dead+Collateral+0.45Wind_Long1R
- Dead+Collateral+0.45Wind_Long2L
- Dead+Collateral+0.45Wind_Long2R
- 0.6Dead+0.6Wind_Left1
- 0.6Dead+0.6Wind_Right1
- 0.6Dead+0.6Wind_Left2
- 0.6Dead+0.6Wind_Right2
- 0.6Dead+0.6Wind_Long1L
- 0.6Dead+0.6Wind_Long2L
- 0.6Dead+0.6Wind_Long2R
- 1.01Dead+1.01Collateral+0.75Seismic_Left
- 1.01Dead+1.01Collateral+0.75Seismic_Right
- 1.01Dead+1.01Collateral+0.75Seismic_LongL
- 1.01Dead+1.01Collateral+0.75Seismic_LongR
- 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_Left
- 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_Right
- 1.01Dead+1.01Collateral+0.53Seismic_Left
- 1.01Dead+1.01Collateral+0.53Seismic_Right
- 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_LongL
- 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_LongR
- 1.01Dead+1.01Collateral+0.53Seismic_LongL
- 1.01Dead+1.01Collateral+0.53Seismic_LongR
- 0.59Dead+0.7Seismic_Left
- 0.59Dead+0.7Seismic_Right

FRAME LINES: C1 C2 C3 C4 C5 C6 C7 C8 C9 C10 C11



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame Line	Col Line	Load Id	Hmax	Vmax	Column Reactions(k)	Bolt Qty	Bolt Dia	Base Plate Width	Base Plate Length	Base Plate Thick	Grout (in)
C1*	1.6	Moment connection, see table.				8	0.750	6.000	15.63	0.375	0.0

C1* Frame lines: C1 C2 C3 C4 C5 C6 C7 C8 C9 C10 C11

RIGID FRAME: REACTIONS FOR FRAME LINE : C1*

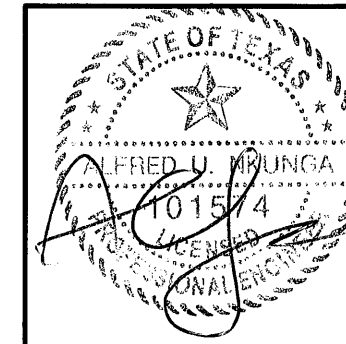
Load Id	Col. Horiz	19.00 (k, f-k)	Vert	Moment
1	0.0	3.4	9.65	
2	-0.1	-1.3	-3.41	
3	-0.3	0.3	4.37	
4	0.3	0.3	-2.98	
5	0.0	1.2	2.88	
6	0.0	0.6	1.17	
7	0.0	2.8	7.96	
8	0.2	-0.6	-5.32	
9	0.2	-0.6	-3.05	
10	0.1	-0.6	-3.34	
11	0.1	-0.6	-4.92	
12	-0.1	-1.1	-2.94	
13	-0.1	-1.1	-2.94	
14	0.2	1.3	0.97	
15	0.2	1.3	0.97	
16	0.1	1.9	3.09	
17	0.1	1.9	4.79	
18	0.1	1.9	4.57	
19	0.1	1.9	3.36	
20	-0.1	1.6	4.87	
21	-0.1	1.6	4.87	
22	0.2	3.4	7.80	
23	0.2	3.4	7.80	
24	0.1	0.2	-1.99	
25	0.1	0.2	-0.29	
26	0.1	0.3	-0.51	
27	0.1	0.3	-1.72	
28	-0.1	-0.1	-0.21	
29	-0.1	-0.1	-0.21	
30	0.2	1.7	2.72	
31	0.2	1.7	2.72	
32	0.2	-0.9	-5.79	
33	0.2	-0.9	-3.52	
34	0.1	-0.8	-3.81	
35	0.1	-0.8	-5.42	
36	-0.1	-1.3	-3.41	
37	0.2	1.1	0.50	
38	0.2	1.1	0.50	
39	-0.1	1.2	4.35	
40	-0.1	1.2	1.47	
41	-0.3	1.2	6.59	
42	0.3	1.2	-0.77	
43	-0.1	2.9	9.08	
44	0.1	2.9	6.90	
45	-0.1	1.2	4.00	
46	0.1	1.2	1.82	
47	-0.2	2.9	10.77	
48	0.2	2.9	5.20	
49	-0.2	1.2	5.69	
50	0.2	1.2	0.12	
51	-0.1	0.3	2.13	
52	0.1	0.3	-0.75	

C1* Frame lines: C2 C3 C4 C5 C6 C7 C8 C9 C10 C11

GENERAL NOTES

- ANCHOR RODS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
- FOUNDATION DESIGN AND ANCHOR RODS LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR ROD SUMMARY TABLE REPORTS THE ROD DIAMETERS.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
- ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8.

JUN 02 2020



ISSUE	DESCRIPTION	DATE	MARK
0	CONSTRUCTION	5/29/20	

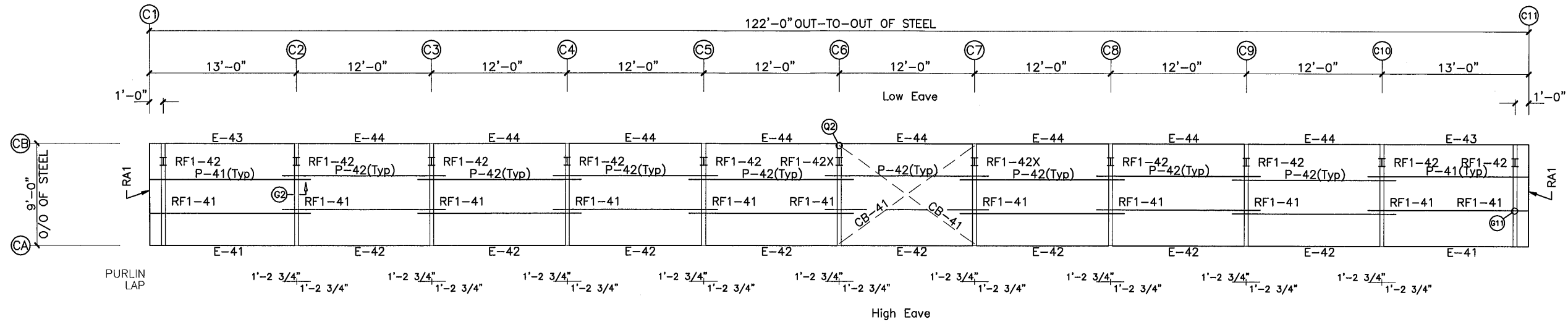


PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

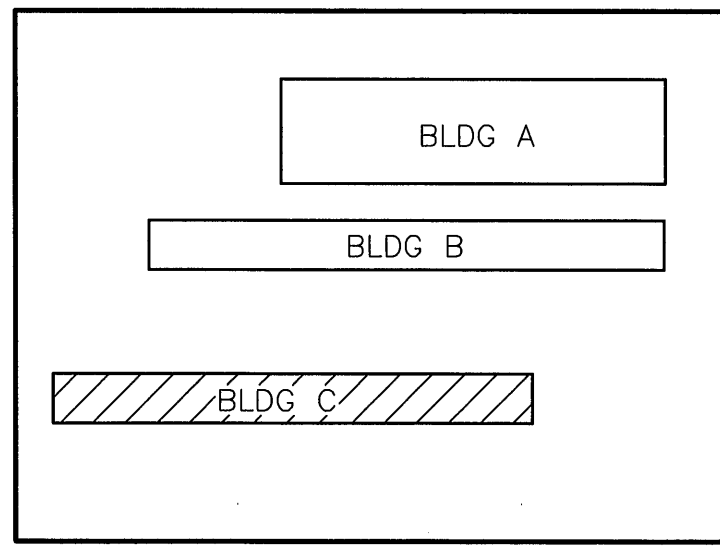
DESCRIPTION:	ANCHOR ROD REACTIONS		
CUSTOMER:	VIA REAL ESTATE, LLC.		
LOCATION:	LAREDO, TX (WEBB CO)		
Detailer	SS	Checker	DS
Designer	KJK		
Job No.	201287C	Sheet	F2
Issue	0		

MEMBER TABLE	
ROOF PLAN	
MARK	PART
P-41	8X25Z16
P-42	8X25Z16
E-41	8ES14-2
E-42	8ES14-2
E-43	8ES14-2
E-44	8ES14-2
CB-41	3/8"CB

NOTE:
Alternate Arrows ▽-△
Up And Down From Bay
To Bay For Purlins To Lap.



ROOF FRAMING PLAN



COLLATERAL LOAD NOTE:
Roof purlin has been designed for the collateral load listed on the cover. The total applied loads due to ceiling panels, ducts, sprinkler distribution lines, electrical equipment, conduit, fireproofing, other piping or mechanical loads cannot exceed this maximum uniform load. Pinnacle Structures, Inc. is not responsible for lateral or longitudinal bracing of suspended members subject to lateral seismic or wind loading.

Loads supported directly from the purlins must have connections through the web of the purlin.

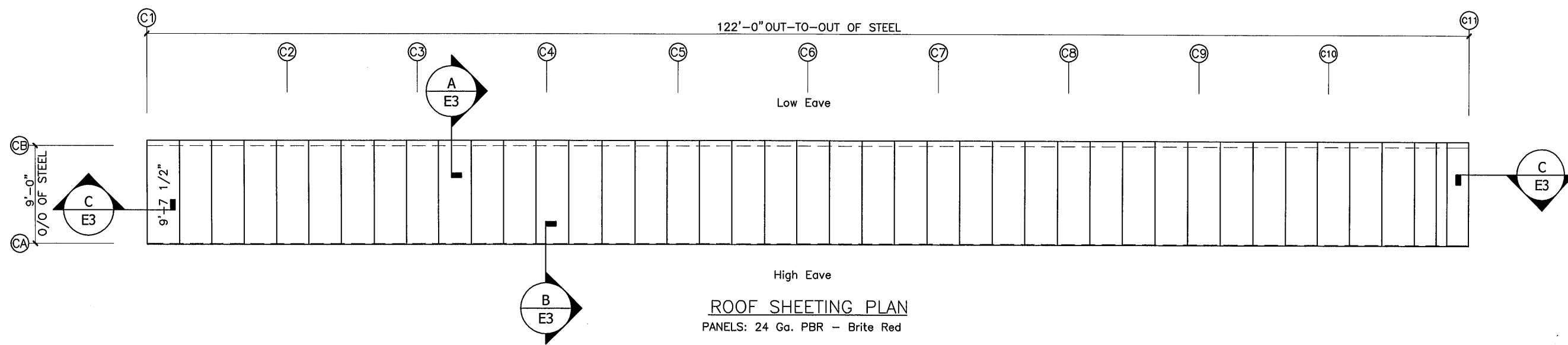
Loads supported between purlins must be supported such that the loads are applied to the webs of the purlins.

FOR PERMITS ONLY

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	5/29/20	



DESCRIPTION:	ROOF FRAMING PLAN		
CUSTOMER:	VIA REAL ESTATE, LLC.		
LOCATION:	LAREDO, TX (WEBB CO)		
Detailer	SS	Checker	DS
Designer	KJK		
Job No.	201287C	Sheet	E1
Issue	P		



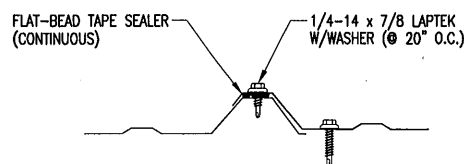
ROOF SHEETING PLAN
 PANELS: 24 Ga. PBR - Brite Red

FOR PERMIT'S ONLY

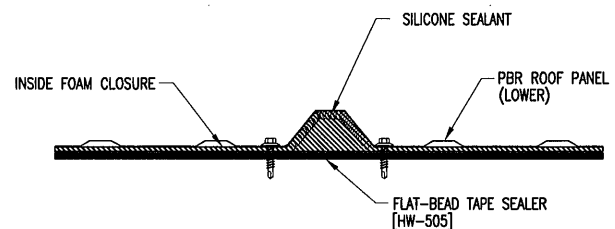
1	2	3	4
ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	5/29/20	



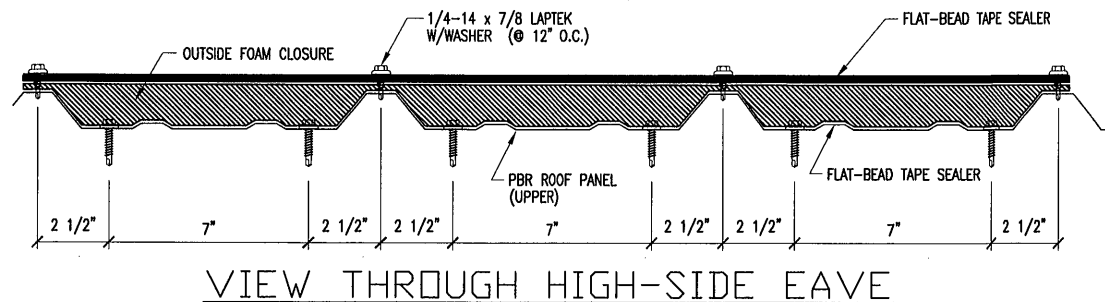
DESCRIPTION:	ROOF SHEETING		
CUSTOMER:	VIA REAL ESTATE, LLC.		
LOCATION:	LAREDO, TX (WEBB CO)		
Detailer	SS	Checker	DS
Designer		Designer	KJK
Job No.	201287C	Sheet	E2
Issue		Issue	P



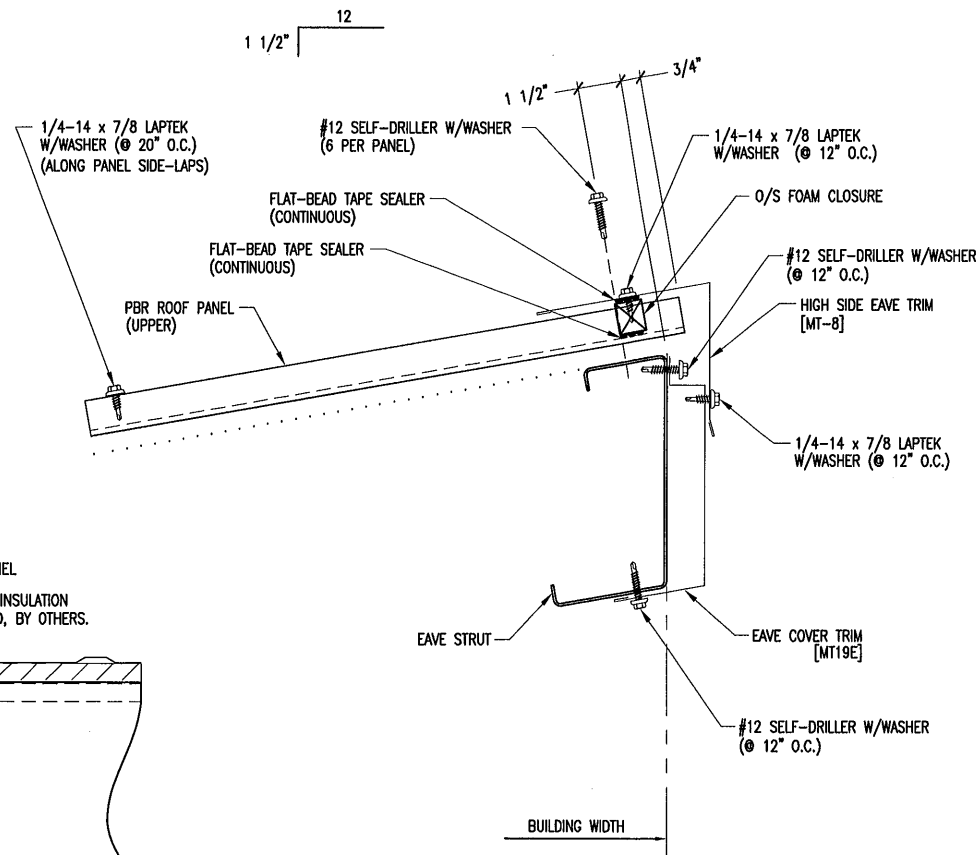
SECTION THROUGH PANEL SIDELAP



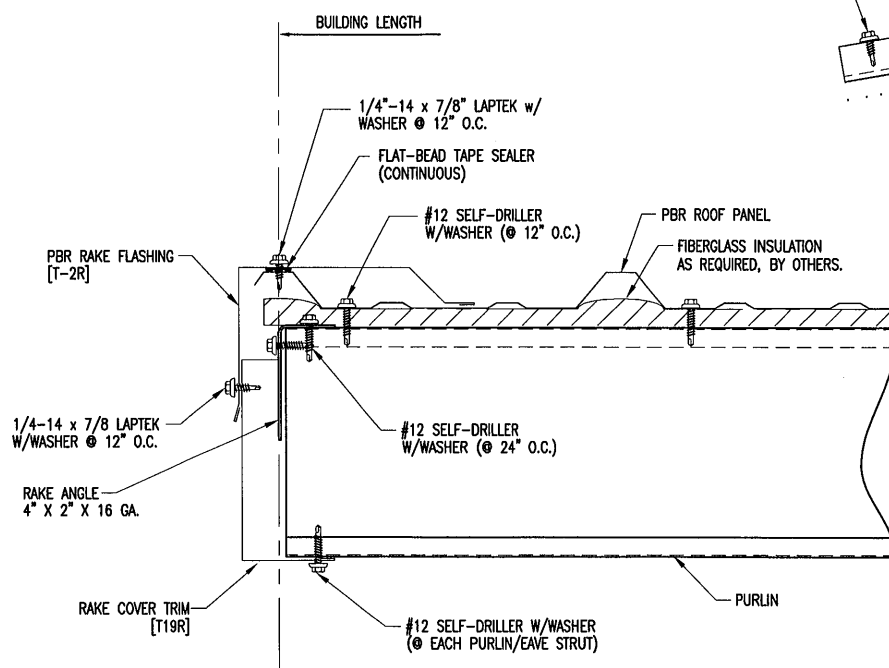
SECTION THROUGH EAVE END



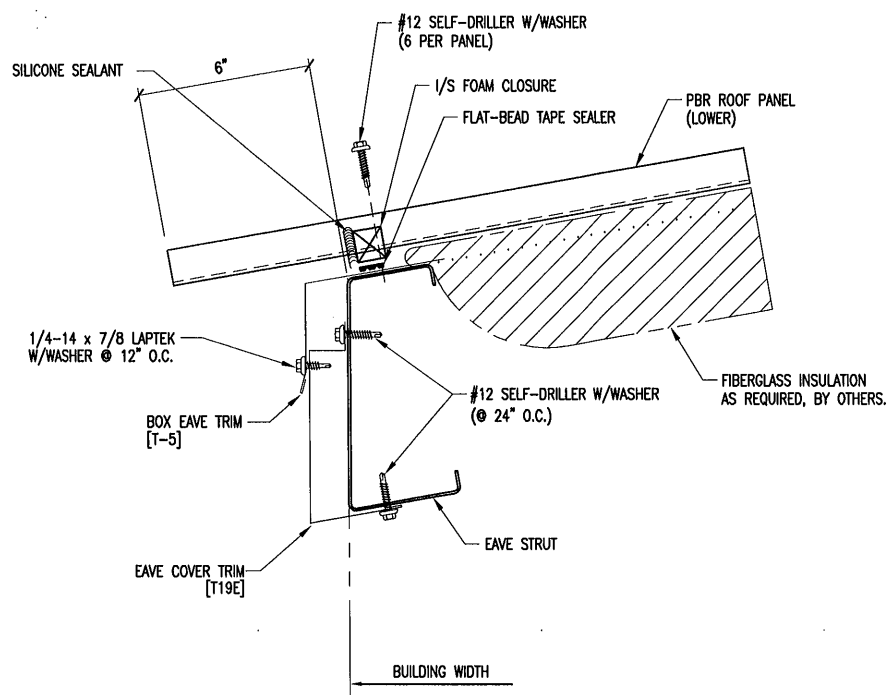
VIEW THROUGH HIGH-SIDE EAVE



B SECTION
E3 DETAIL



C SECTION
E3 DETAIL



A SECTION
E3 DETAIL

FOR PERMITS ONLY

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	5/29/20	

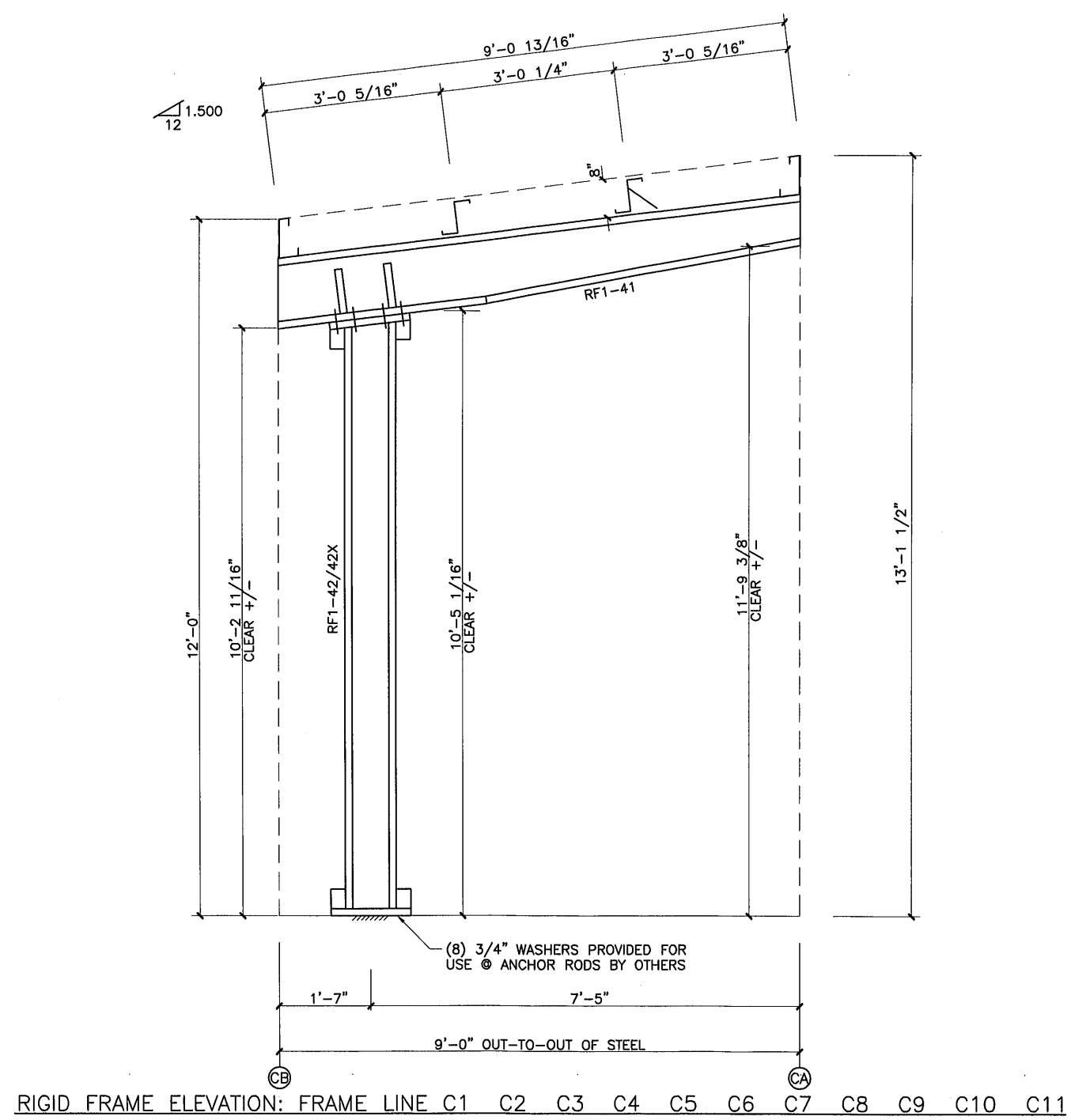


DESCRIPTION:	PBR ROOF PANEL DETAILS		
CUSTOMER:	VIA REAL ESTATE, LLC.		
LOCATION:	LAREDO, TX (WEBB CO)		
Detailer	SS	Checker	DS
Designer	KJK		
Job No.	201287C	Sheet	E3
Issue	P		

CAP PLATE BOLTS				
Mark	Qty	Type	Dia	Length
RF1-42	8	A325	5/8"	1 1/2"

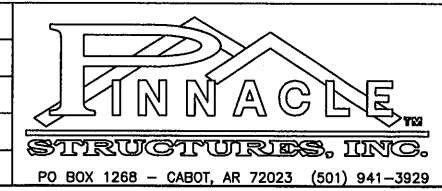
FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxxA(1): xx=length(in)
 A - L2X2X1/8

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
	Start	End	Thick	Length		
RF1-41	12.5	12.5	0.135	43.3	6 x 1/4" x 108.8	6 x 3/8" x 43.3
RF1-42	12.5	7.5	0.135	67.1		6 x 1/4" x 66.3

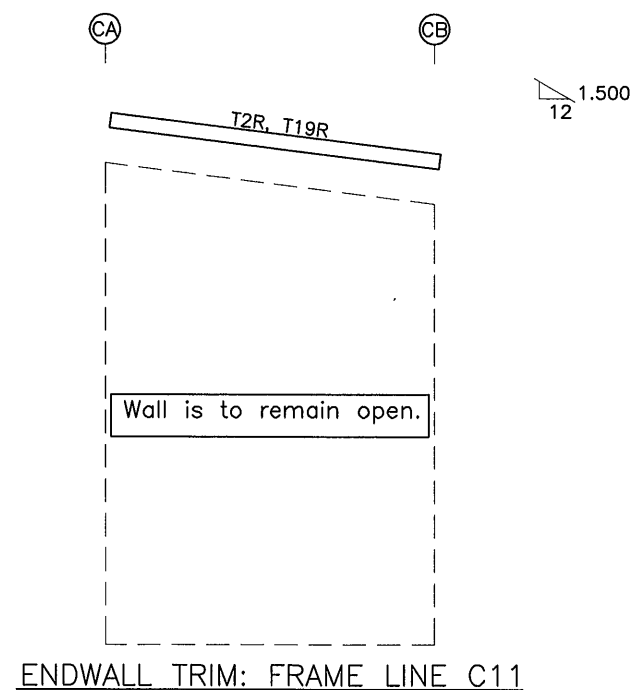
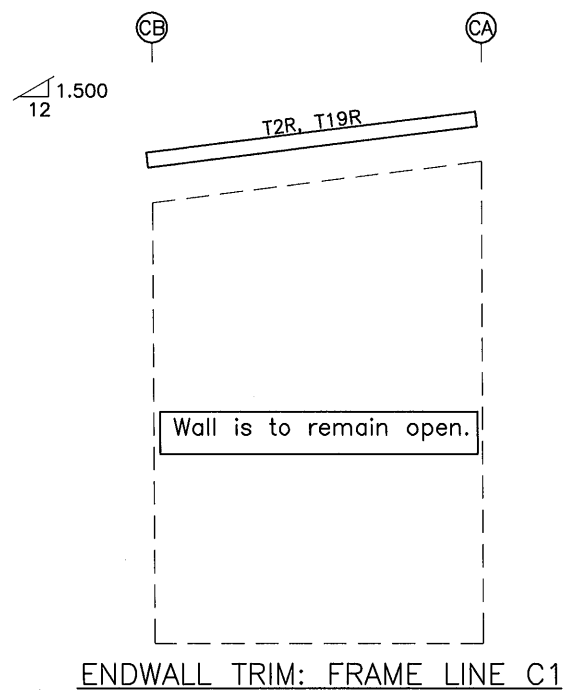
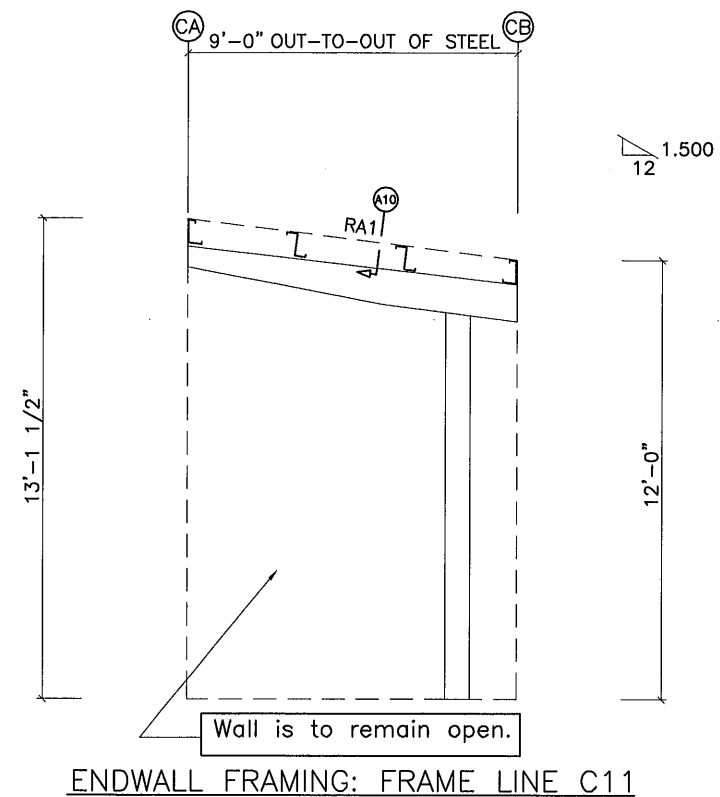
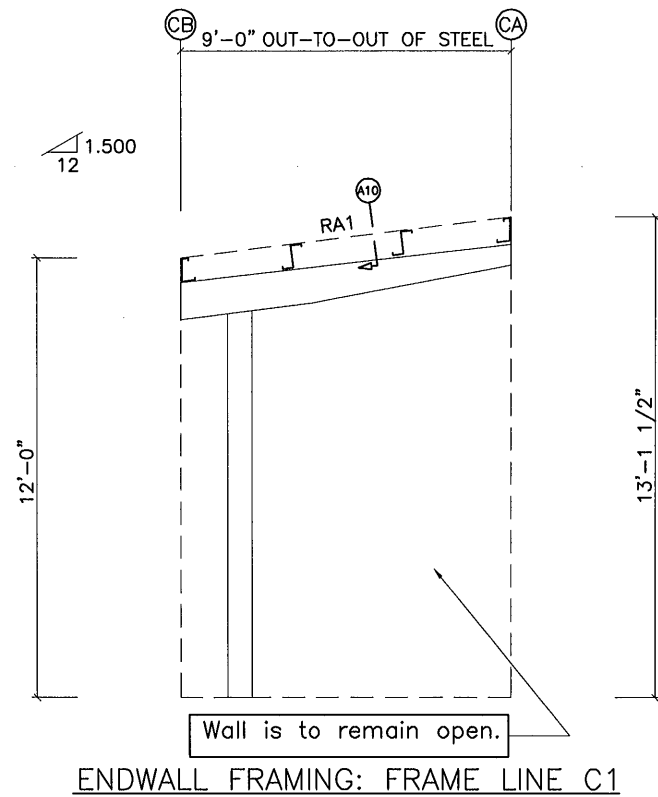


FOR PERMITS ONLY

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	5/29/20	



DESCRIPTION: RIGID FRAME ELEVATION		
CUSTOMER: VIA REAL ESTATE, LLC.		
LOCATION: LAREDO, TX (WEBB CO)		
Detailer SS	Checker DS	Designer KJK
Job No. 201287C	Sheet E4	Issue P



PERMITS ONLY

GENERAL NOTES:

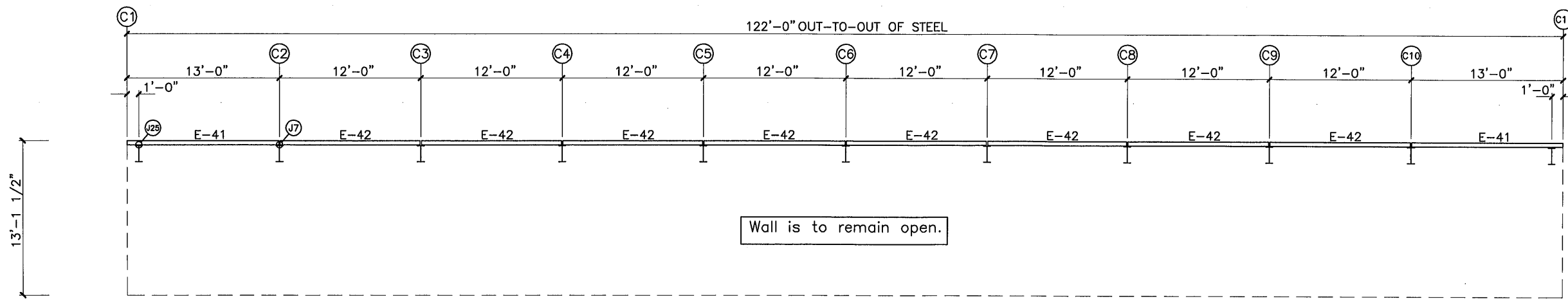
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girls in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	5/29/20	

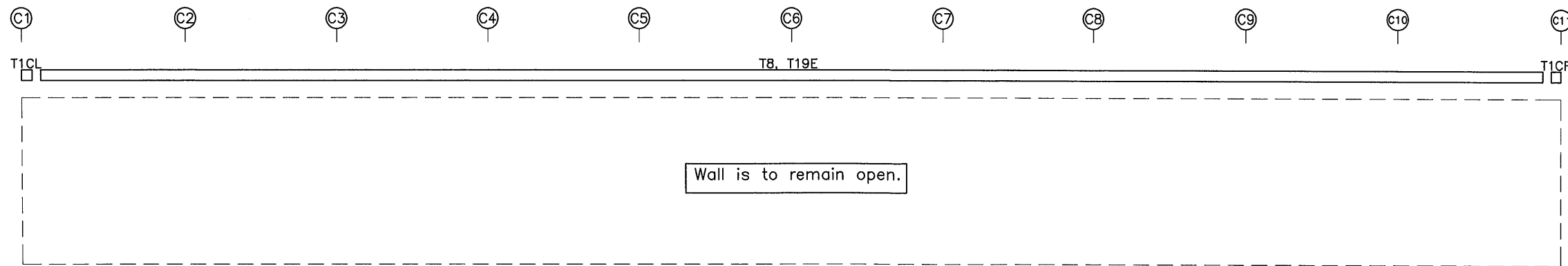


DESCRIPTION:	ENDWALL FRAMING		
CUSTOMER:	VIA REAL ESTATE, LLC.		
LOCATION:	LAREDO, TX (WEBB CO)		
Detailer	SS	Checker	DS
Designer		Designer	KJK
Job No.	201287C	Sheet	E5
Issue		Issue	P

MEMBER TABLE	
FRAME LINE CA	
MARK	PART
E-41	8ES14-2
E-42	8ES14-2



SIDEWALL FRAMING: FRAME LINE CA



SIDEWALL TRIM: FRAME LINE CA

FOR PERMITS ONLY

GENERAL NOTES:

1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

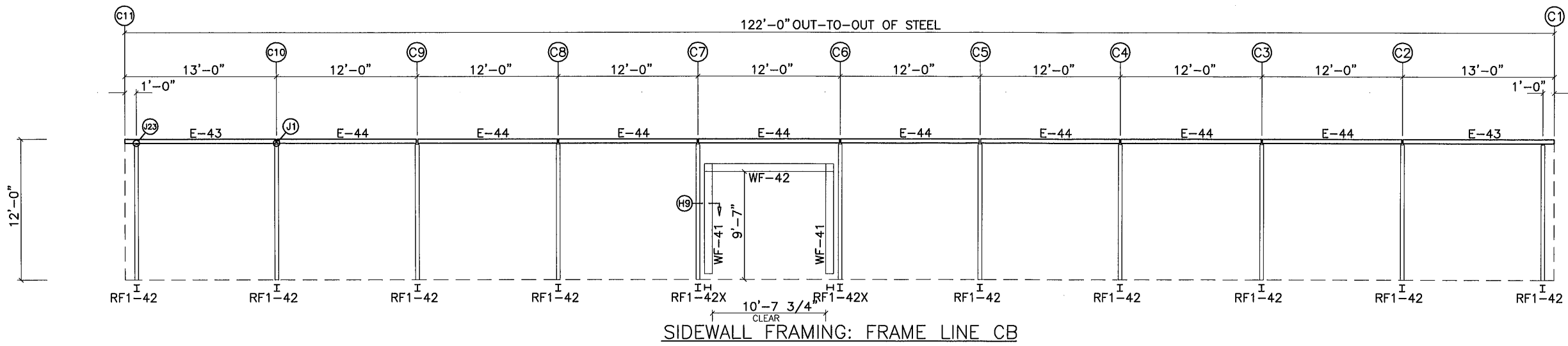
ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	5/29/20	



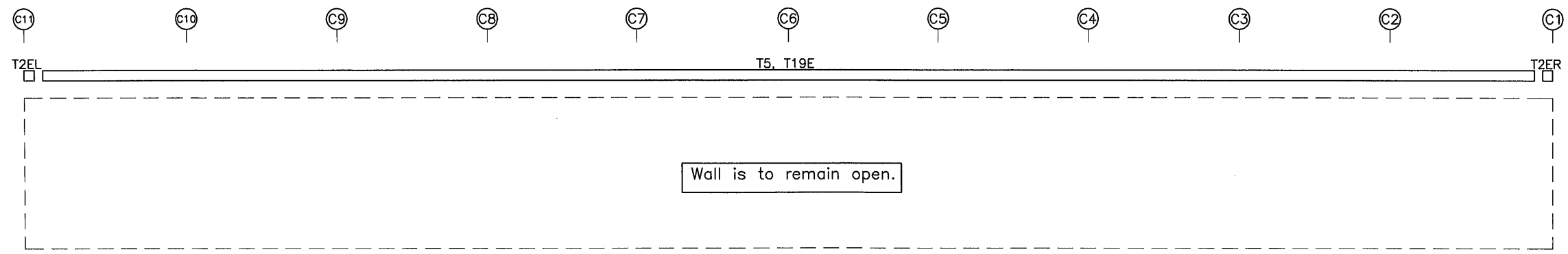
DESCRIPTION:	SIDEWALL FRAMING		
CUSTOMER:	VIA REAL ESTATE, LLC.		
LOCATION:	LAREDO, TX (WEBB CO)		
Detailer	SS	Checker	DS
Designer	KJK		
Job No.	201287C	Sheet	E6
Issue	P		

BOLT TABLE				
FRAME LINE CB				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-41 - WF-42	8	A325	3/4"	1 3/4"
WF-41 - RF1-42X	6	A325	5/8"	1 1/2"

MEMBER TABLE	
FRAME LINE CB	
MARK	PART
WF-41	W08641
WF-42	W08641
E-43	8ES14-2
E-44	8ES14-2



SIDEWALL FRAMING: FRAME LINE CB



SIDEWALL TRIM: FRAME LINE CB

GENERAL NOTES:
 1. Pinnacle standard trim lap is 3 inches max.
 2. Pinnacle pre-cuts wall panels at factory located openings as required.
 3. Slot girts in field for cable passage at flush walls as required.
 4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
P	PERMIT	5/29/20	



DESCRIPTION:	SIDEWALL FRAMING		
CUSTOMER:	VIA REAL ESTATE, LLC.		
LOCATION:	LAREDO, TX (WEBB CO)		
Detailer	SS	Checker	DS
Designer	KJK		
Job No.	201287C	Sheet	E7
Issue	P		

FOR PERMIT ONLY